



# **Understanding and Using the Mechanistic Empirical Pavement Design Guide (MEPDG)**

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# AGENDA

**My view on DARWin-ME / MEPDG**

**Pavement Design Procedure**

**Example – Monroe Bypass, Charlotte NC**

**Initial Costs Savings**

**Life Cycle Costs Savings**

**Stressing the Results**

**Other issues**

# DARWIN-ME / MECHANISTIC EMPIRICAL PAVEMENT DESIGN GUIDE

New design procedure adopted by AASHTO in April 2011 as its Pavement Design Guide

## DARWin-ME / MEPDG Facts

State-of-the-practice design procedure based on advanced models & actual field data collected across the US

- New and rehabilitated pavements
- Calibrated with more than 2,400 asphalt and concrete pavement test sections across the U.S. and Canada, ranging in ages up to approximately 37 years

Uses mechanistic-empirical principles that account for:

- Traffic
- Climate
- Materials
- Proposed structure (layer thicknesses and features)

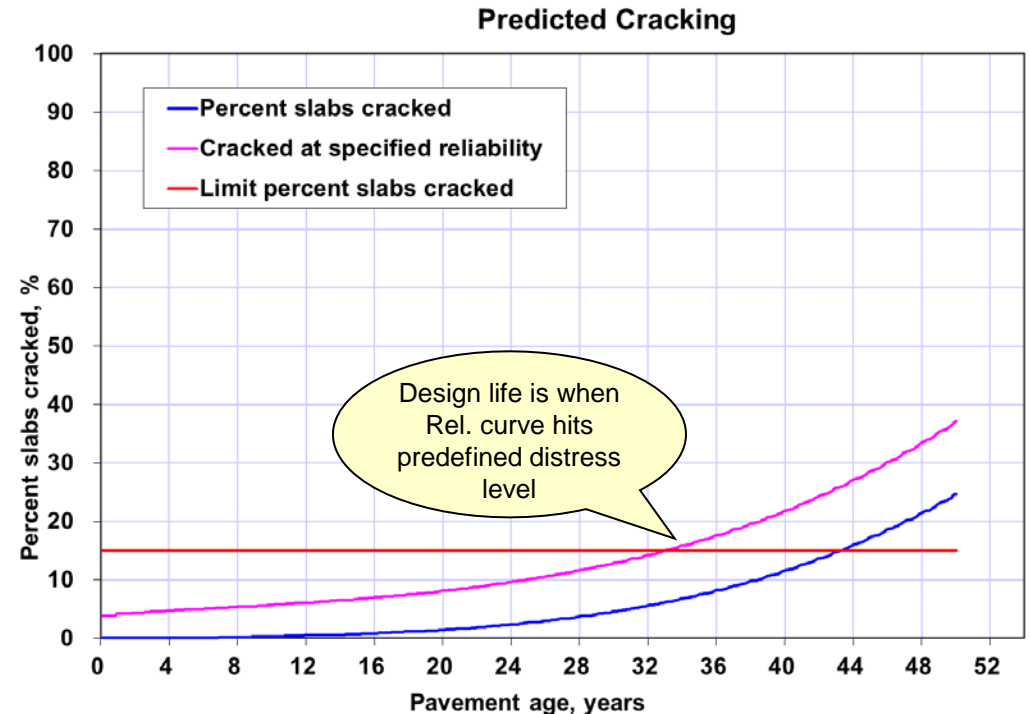
Provides performance estimates during the analysis period

- Criteria = IRI, cracked slabs, faulting, punchouts, crack spacing, load transfer, cumulative damage
- All other procedures (eg AASHTO 93) only provides thickness (no performance)

JPCP Distress 

CRCP Distress 

## DARWin-ME / MEPDG Pavement Performance Curve



**Red Line - Defined Distress Limit.** When major rehabilitation is needed (i.e. patching & DG or overlay).

**Blue Line -** The actual (most likely) level of distresses predicted

**Magenta Line -** The predicted distresses at the given reliability level (i.e. 90%). Designs are based on when this line hits the defined distress limit

# FOR CONCRETE PAVEMENTS

## MEPDG/DARWin-ME can remove Historic Over-design

Pavements designs (thicknesses & features) are based on old design models

- Concrete is assumed to last the design life, without rehabilitation
- Asphalt is designed with rehabilitation activities in mind
  - Lower initial costs, but higher rehabilitation costs

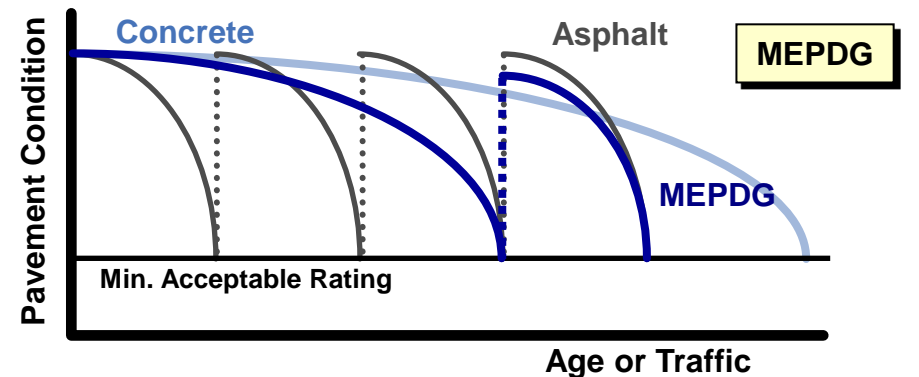
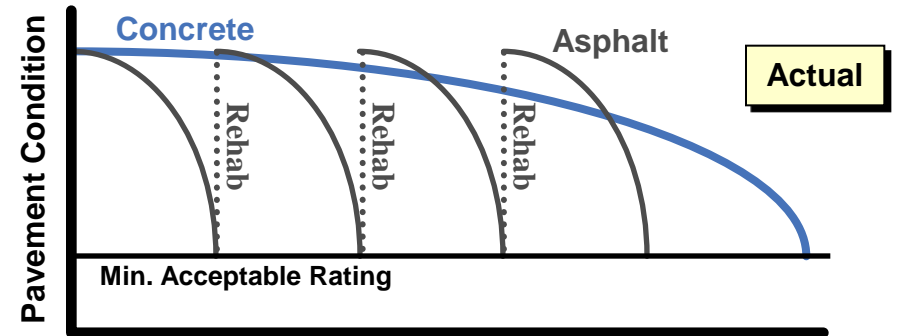
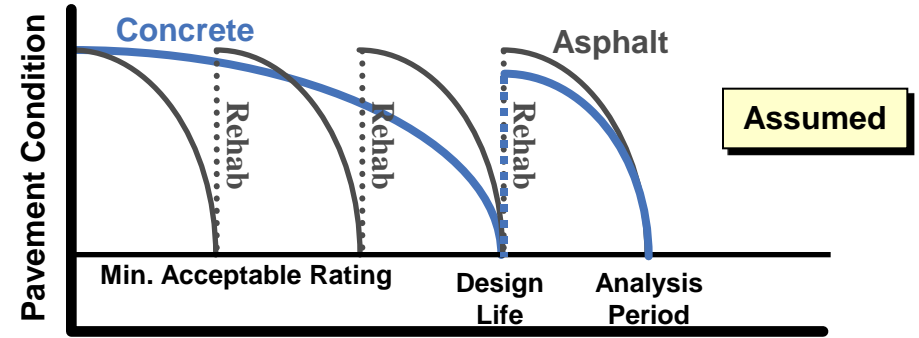
LTPP and other data shows that most concrete pavements have carried many more loads than for which they were designed

- While increased performance is good, it comes at a cost that may be beyond the DOT's budget

CA Data 

MEPDG designs uses newer models to match the pavement design life to the required performance life

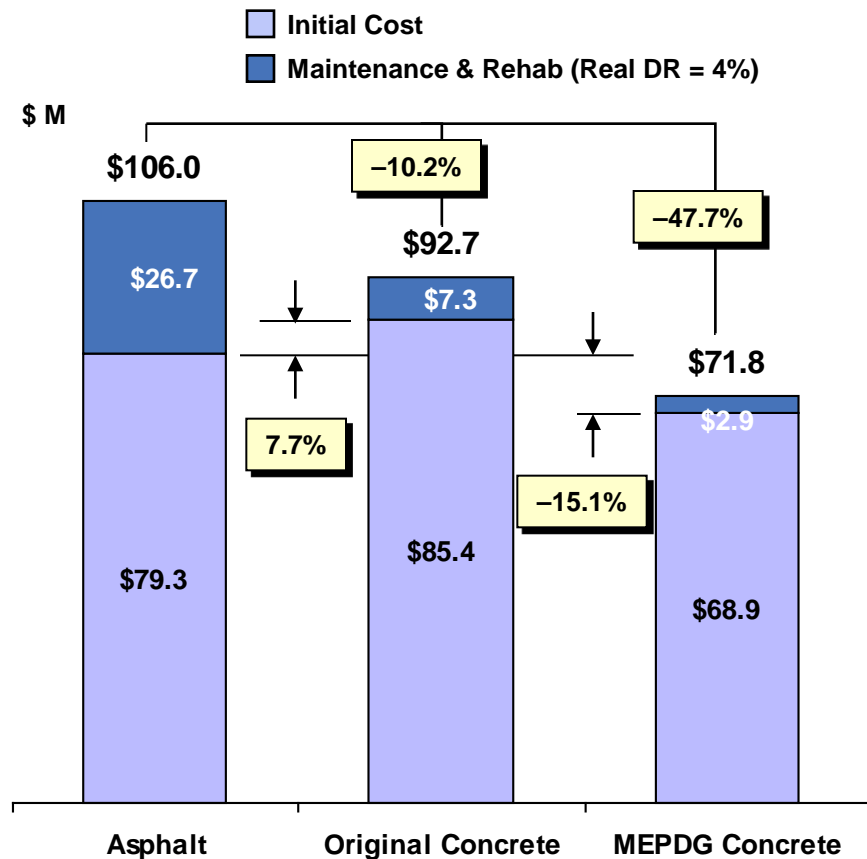
- Reduces the initial costs
- without sacrificing life cycle costs



# DARWIN-ME / MEPDG DESIGNS CAN DELIVER SUBSTANTIAL SAVINGS TO A PROJECT

## DARWin-ME / MEPDG allows for design optimization

### Lower Initial and Long Term Cost



### Optimization Opportunities

- Pavement Design
  - Thickness
  - Shoulders
  - Steel content
- Concrete Mix Design
- Base Design
- Soil Stabilization

### Benefits to optimizing pavement design

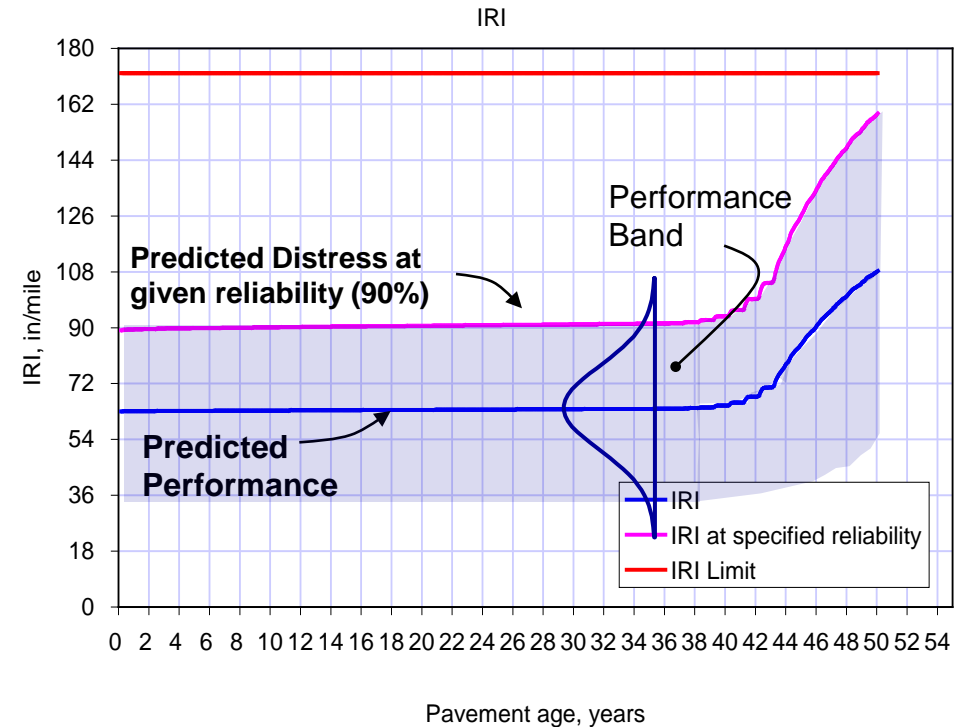
- Decreased Concrete & Steel Requirements
- Decreased construction times (due to less materials)
- Decreased material delivery costs and cement and steel manufacturing requirements
  - fewer truck loads of concrete, cement/FA, aggregate, steel, etc
- Improved pavement sustainability due to lower material requirements

**Optimizing can lower both the initial costs and life cycle costs of the project**

# UNDERSTANDING THAT IT PROVIDES BETTER DESIGNS, DARWIN-ME / MEPDG IS JUST A TOOL

## Predicted Performance Curves & Pavement Designs

- Performance curves are estimates
  - It is not an exact answer
  - DARWIN-ME / MEPDG gives a distribution of what the actual performance could be
- There is no correct pavement design
  - There are many pavement designs that will work
  - MEPDG/DARWin-ME allows for comparisons and evaluation of different design features
- Performance estimates help determine the when and what rehabilitation activities to perform



## Need to use with LCCA

- DARWin-ME/MEPDG should NOT be used by itself – needs to be combined with a LCCA
  - The user needs to develop a pavement design that meets the owners budget
  - It is easy to develop designs to meet a given performance criteria.
  - Need to find the design that balances the initial costs, life cycle costs & performance

**DARWIN-ME / MEPDG is a tool that helps identify and manage risks;  
and should be used with LCCA to quantify those risks**

# WHILE THE CONSENSUS IS THAT DARWIN-ME / MEPDG A BIG IMPROVEMENT OVER CURRENT DESIGN PROCESSES

There are many concerns with implementation

Issues & Concerns	
Too complicated Too many inputs	<ul style="list-style-type: none"> <li>• Over 200 design variables</li> <li>• Assembling the data required to run this program takes considerable time</li> <li>• It requires too much training – minimum of one week, assuming the person being trained is an experienced pavement or materials engineer</li> <li>• Traffic Data is too voluminous and pavement engineers do not know what reasonable values are</li> </ul>
Material Properties	<ul style="list-style-type: none"> <li>• Many inputs are not commonly encountered by materials or pavement engineers                             <ul style="list-style-type: none"> <li>• hydraulic conductivity, thermal conductivity, etc.</li> </ul> </li> <li>• Does not handle some of the more common materials used by “my DOT”</li> <li>• Many values that have a significant impact on the design will not be known until during or after construction                             <ul style="list-style-type: none"> <li>• Specification only defines the range of acceptable values for materials – not the actual value</li> <li>• Substantial testing of layer materials is required</li> <li>• Mix design is not known, construction month not known, etc.</li> </ul> </li> <li>• Concerns with the sensitivity of the inputs and the effects of different parameters on predicted distresses</li> </ul>
Calibration	<ul style="list-style-type: none"> <li>• It is not calibrated to local conditions – the performance models used are calibrated using limited national databases and does not take into account local materials, traffic, and environmental conditions.</li> <li>• Calibration for the local materials and conditions in that state can cost \$500,000 to \$1,000,000 and may take six months to a year.</li> </ul>
Others	<ul style="list-style-type: none"> <li>• Only evaluates a proposed design – there is no real way to suggest layer types and thicknesses.</li> <li>• Asphalt side is not ready</li> <li>• Too Expensive</li> </ul>

While these are a concern, I use inputs that are reasonable and address these impacts by doing a “Life Cycle Cost sensitivity analysis”

# AGENDA

**My view on DARWin-ME / MEPDG**

**Pavement Design Procedure**

**Example – Monroe Bypass, Charlotte NC**

**Initial Costs Savings**

**Life Cycle Costs Savings**

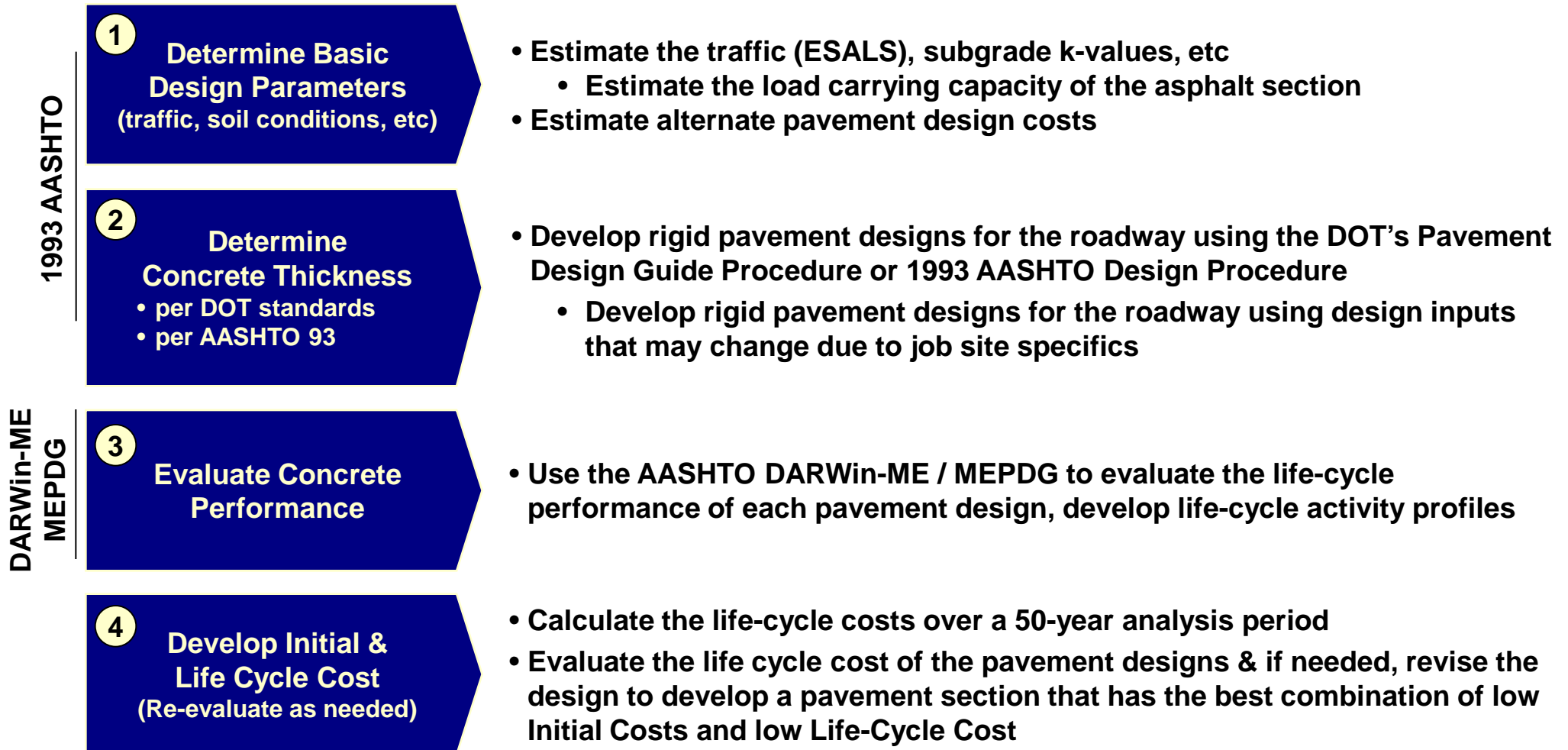
**Stressing the Results**

**Other issues**



# OPTIMIZING DESIGN STARTS WITH THE STATE DOT PROCEDURE

## And checks & refines designs with the AASHTO DARWin-ME / MEPDG



# REHABILITATION STRATEGIES ARE CHOSEN BASED ON THE MEPDG / DARWIN-ME PREDICTION CURVES

	Concrete Pavement Preservation	Asphalt Overlays
Activities	<ul style="list-style-type: none"> <li>• A set of activities used early in the life of the pavement to repair isolated area of deteriorated pavement                             <ul style="list-style-type: none"> <li>• Concrete Patching (% of pavement surface)</li> <li>• Diamond grinding (% of pavement surface)                                     <ul style="list-style-type: none"> <li>- Can be repeated up to 3 times</li> </ul> </li> </ul> </li> <li>• Typical life is ~ 10 years</li> </ul>	<ul style="list-style-type: none"> <li>• 2" to 4+'' asphalt overlay                             <ul style="list-style-type: none"> <li>• May include milling of existing pavement surface</li> <li>• Can be repeated many times</li> </ul> </li> <li>• Typical life is ~ 10 years</li> </ul>
Cost Impact	<ul style="list-style-type: none"> <li>• Concrete Patch <math>\approx</math> \$2000 / patch</li> <li>• Diamond Grinding <math>\approx</math> \$3 – \$4 / SY</li> </ul>	<ul style="list-style-type: none"> <li>• Asphalt Overlay <math>\approx</math> \$3.50 – \$4.50/SY/in</li> <li>• Milling <math>\approx</math> \$1 – \$4 / SY</li> </ul>



Concrete Patch  $\approx$  \$2000 / patch



Diamond Grinding  $\approx$  \$3 – \$4 / SY



Milling  $\approx$  \$1 – \$4 / SY

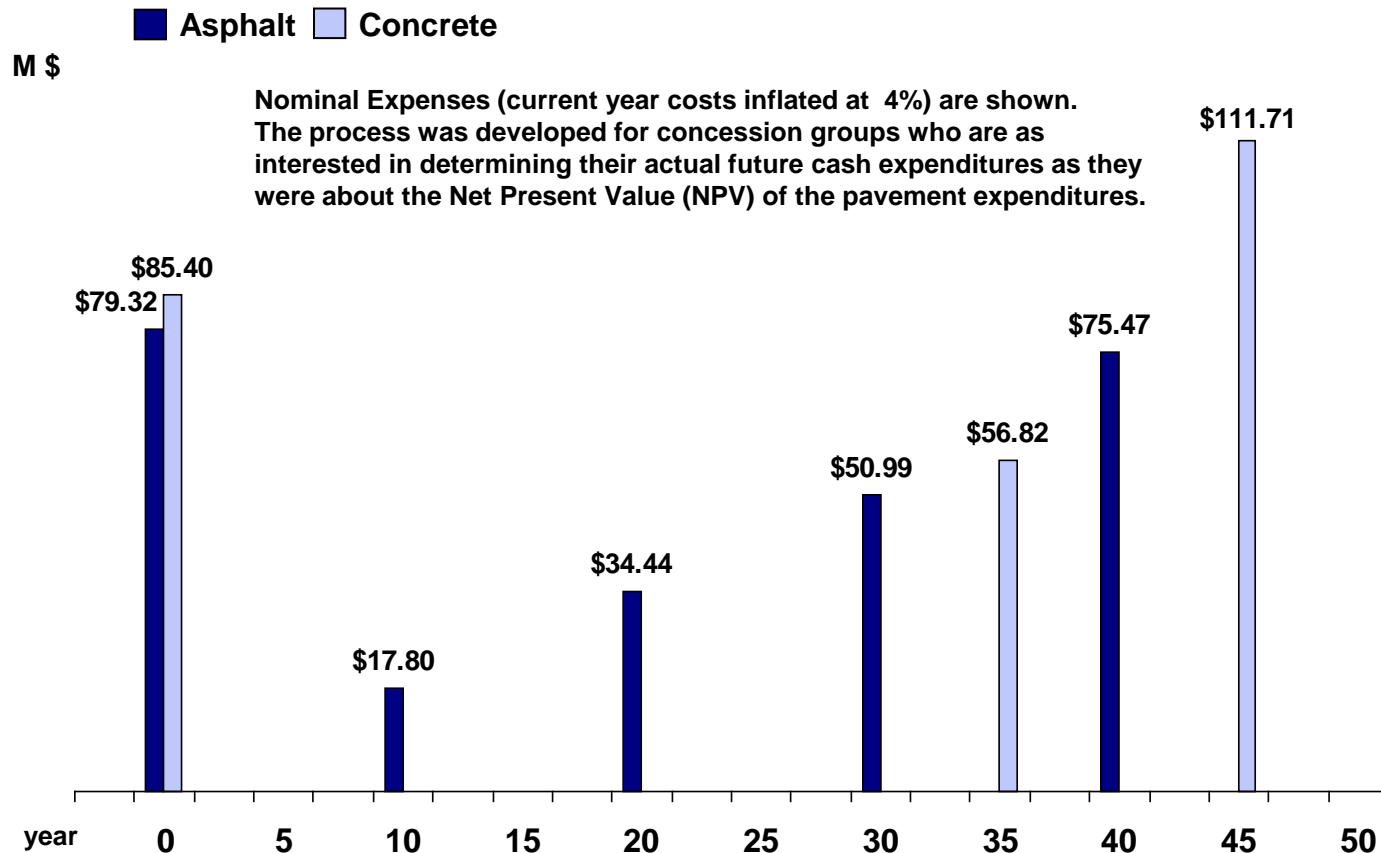


Asphalt Overlay  $\approx$  \$3.50 – \$4.50/SY/in

**Choice between CPP and AC overlay is based on cost and estimated performance**

# LIFE-CYCLE COSTS ANALYSIS IS USED TO ASSESS THE TOTAL “COSTS OF OWNERSHIP” OVER THE LIFE OF AN ASSET

## Nominal Expenditures by Pavement Type (\$ M)



Life-cycle cost analysis (LCCA) is used to make better pavement investment decisions

- Evaluates the trade offs between cost and performance

### Initial costs

- Pavement, base, and subgrade stabilization materials and labor

### Rehabilitation costs

- Timing & activities based on MEPDG/DARWIN-ME
- Also include
  - Other Incidental Costs such as striping, mobilization, etc. (20% of material costs)
  - Traffic Control (5% of material cost)
  - Engineering & Inspection (5% of material cost)

**The decisions made today commit future resources (dollars & time) for maintenance & rehabilitation**

# FINAL DESIGN IS FOUND BY ITERATING FEATURES AND BALANCING INITIAL COSTS, LIFE CYCLE COSTS & PERFORMANCE

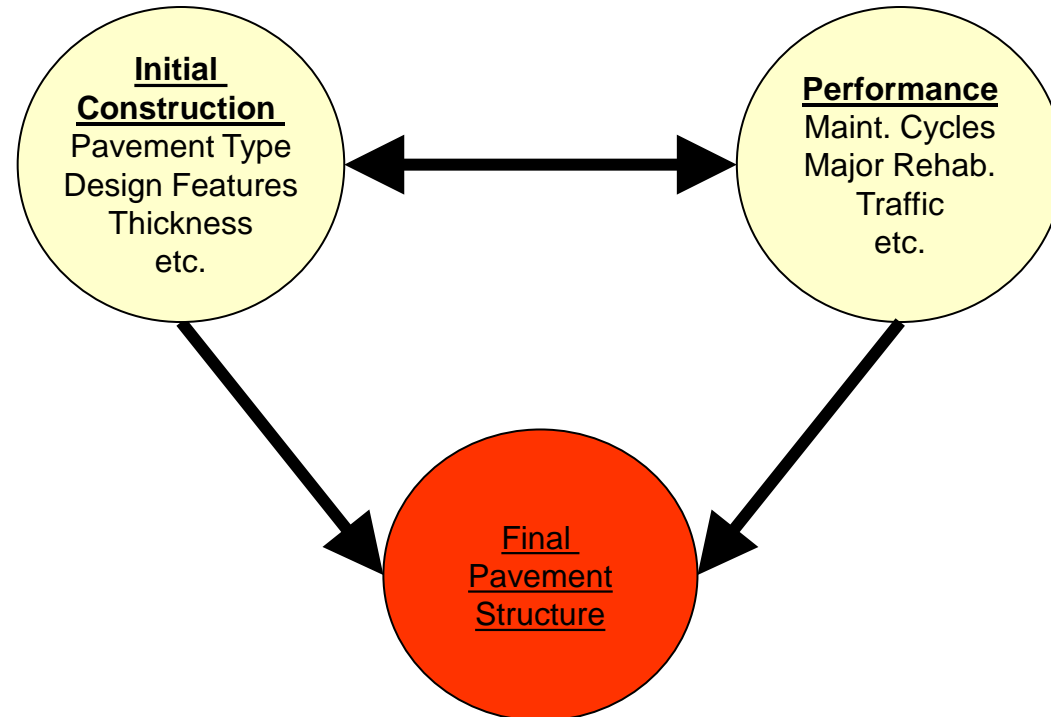
## Use both Engineering and Economics

### Engineering

- Develop the pavement layer thicknesses and features for:
  - a given traffic level
  - the environmental conditions of the project
  - The subgrade properties of the project.
- Develop different structures for different periods of time
- Estimate the predicted performance of each alternate

### Economic

- Initial costs associated with each structure.
- Life cycle costs associated with the maintenance / rehabilitation costs of each alternate to keep the pavement performing during the analysis period



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**Example – Monroe Bypass, Charlotte NC**

**Initial Costs Savings**

**Life Cycle Costs Savings**

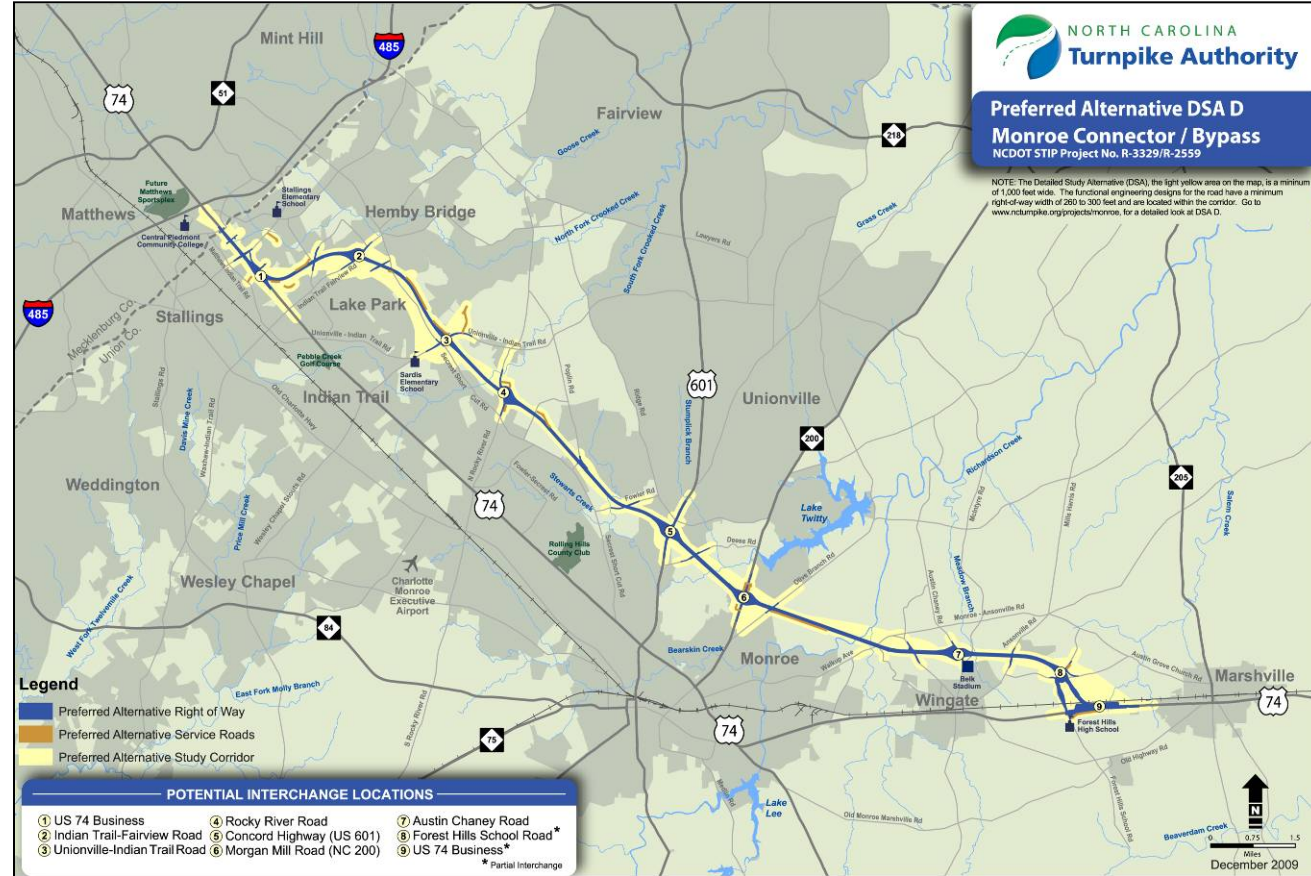
**Stressing the Results**

**Other issues**

# MONROE PARKWAY IS NEW ROAD NEAR CHARLOTTE NC

From US 74 at I-485 in eastern Mecklenburg County to US 74 near the Town of Marshville

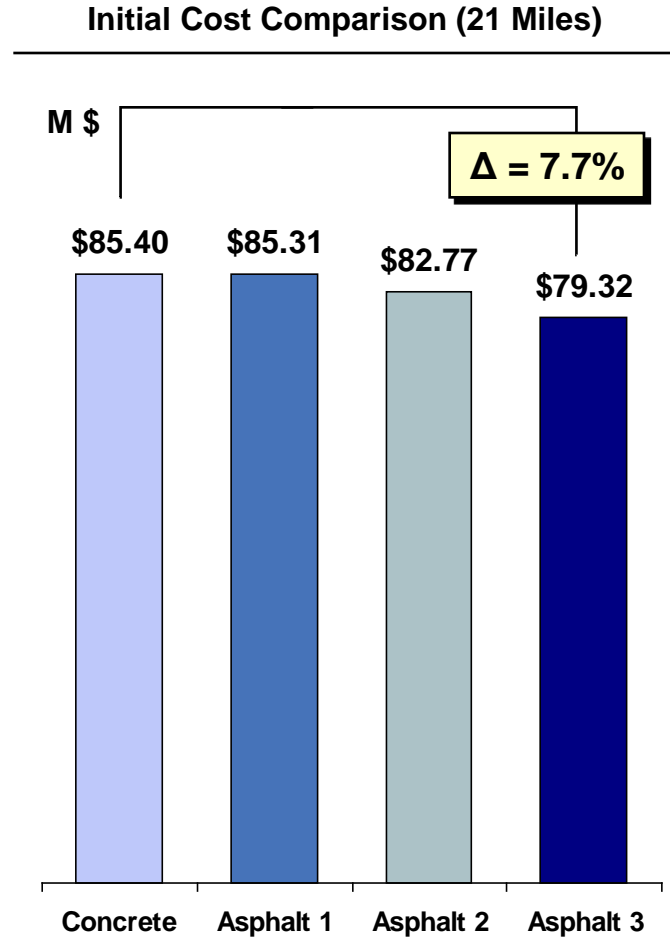
- Project owner: North Carolina Turnpike Authority (NCTA)
- Preliminary cost estimate ~ \$520 M  
Project was let as Design-Build with alternate pavement designs (asphalt or concrete)
- Length is approximately 21 miles
- Estimated Traffic:<sup>1</sup>
  - Yr 2015 – ADT = 35,600
  - Yr 2030 – ADT = 56,600
    - % Duals = 1 % TTST = 2%
    - Growth = 3.14%
  - 20-yr F-ESALS<sup>2</sup> = 7.74 M
  - 30-yr R-ESALS<sup>2</sup> = 18.0 M



1. NCTA – Proposed Monroe Connector/Bypass Preliminary Traffic and Revenue Study – 2009 Update  
 2. F-ESALS based on Dual TF = 0.35, TTST TF = 1.15, Lane Distribution Factor = 0.8 (3 lanes / direction)  
 R-ESALS based on Dual TF = 0.3, TTST TF = 1.6, Lane Distribution Factor = 0.8 (3 lanes / direction)

# NCTA PROPOSED PAVEMENT DESIGNS IN THE DRAFT RFP

Concrete	Asphalt Design #1	Asphalt Design #2	Asphalt Design #3
12.5" PCC Jointed w/ Dowels	3" Asphalt Surface (S12.5D PG 76-22)	3" Asphalt Surface (S12.5D PG 76-22)	3" Asphalt Surface (S12.5D PG 76-22)
3" PATB	4" Asphalt Inter. (I19.0D PG 70-22)	4" Asphalt Inter. (I19.0D PG 70-22)	4.0" Asphalt Inter. (I19.0D PG 70-22)
1.25" Sur Coarse (SF9.5A)	10.0" Asphalt Bases (B25.0C PG 64-22)	5.5" Asphalt Bases (B25.0C PG 64-22)	4.0" Asphalt Bases (B25.0C PG 64-22)
7" Cement Stab. SG or 8" Lime Stab. SG	7" Cement Stab. SG or 8" Lime Stab. SG	10" Aggregate Base	8.0" Cement Treat Aggr Base
		7" Cement Stab. SG or 8" Lime Stab. SG	7" Cement Stab. SG or 8" Lime Stab. SG
Subgrade	Subgrade	Subgrade	Subgrade



Concrete has 3 Shoulder options

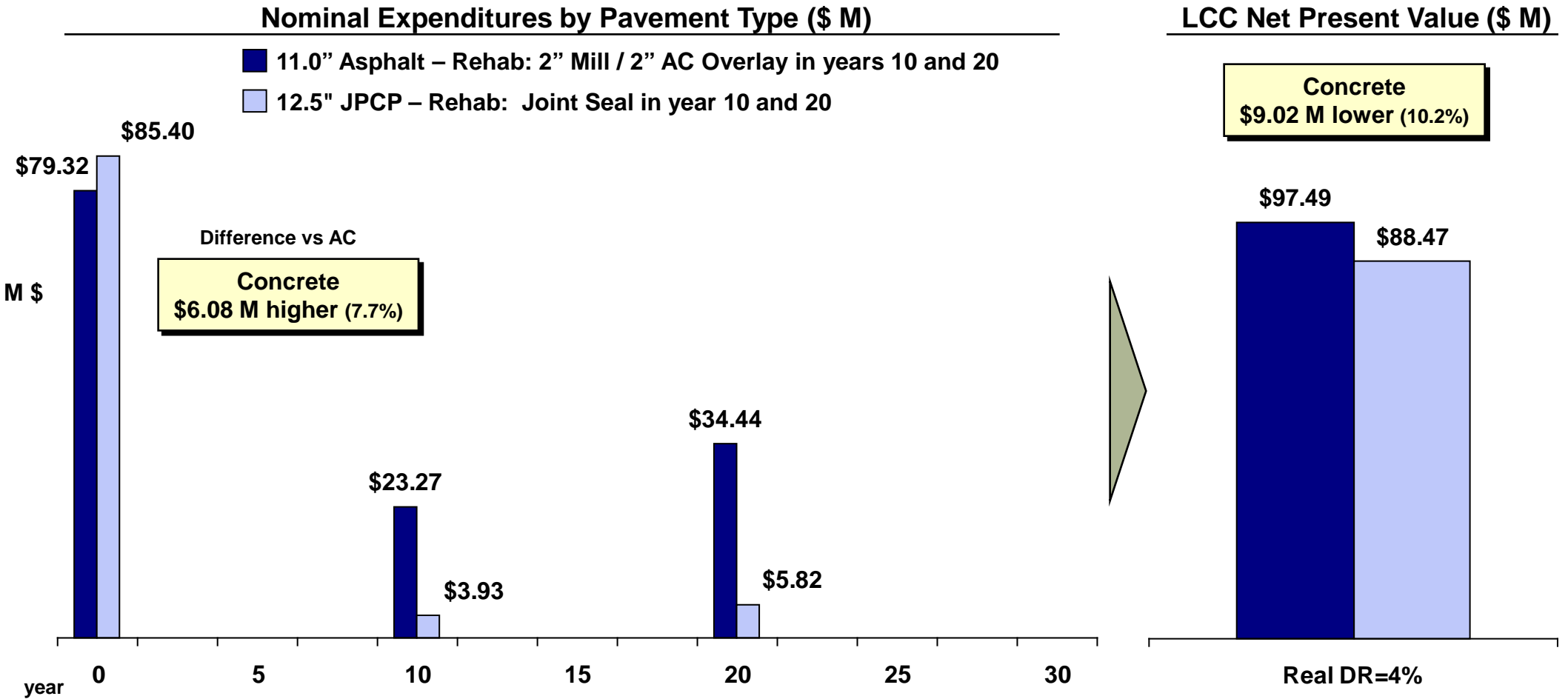
- 12.5" Asphalt
- 9.5" Concrete
- 8" RCC / 5.75" Agg Base

These are designs for Station 183+75 to Station 830+00 – largest portion of the Project. There were two other section with different designs.

Costs include 3 lanes, 2 directions - Pavement, base, and subgrade stabilization materials and labor.

12.5" Conc Pvmt = \$44.39/SY: AC Surface = \$35.17/ton, AC Inter = \$37.74/ton, AC Base = \$38.56/ton, Liq AC = \$503.21/ton

# STANDARD LCCA PROCEDURES USED BY NCDOT SLIGHTLY FAVORS CONCRETE



**Based on NCDOT Process - selection would be concrete**  
 Question 1: is 7.7% saving now is worth more than 10.2% savings in 30 years?  
 Question 2: How accurate are the rehabilitation schedules?

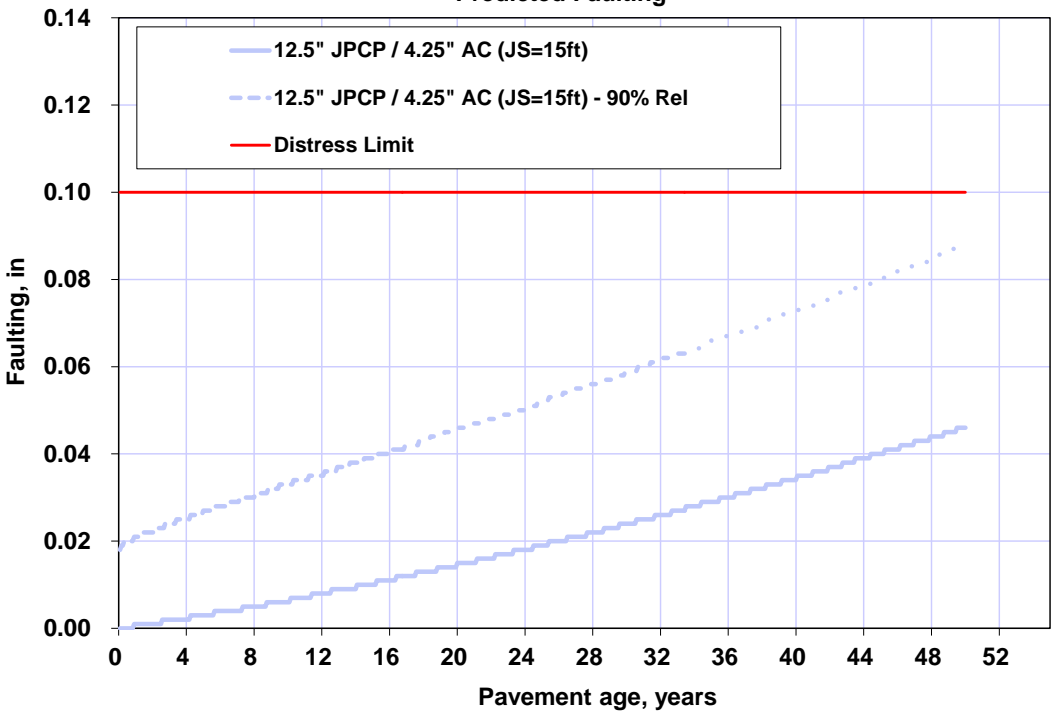
Costs for 21 miles, 3 lanes plus Shoulders. Initial Costs include Pavement, base, and subgrade stabilization materials and labor  
 Rehabilitation costs –Activities based on NCDOT Schedules



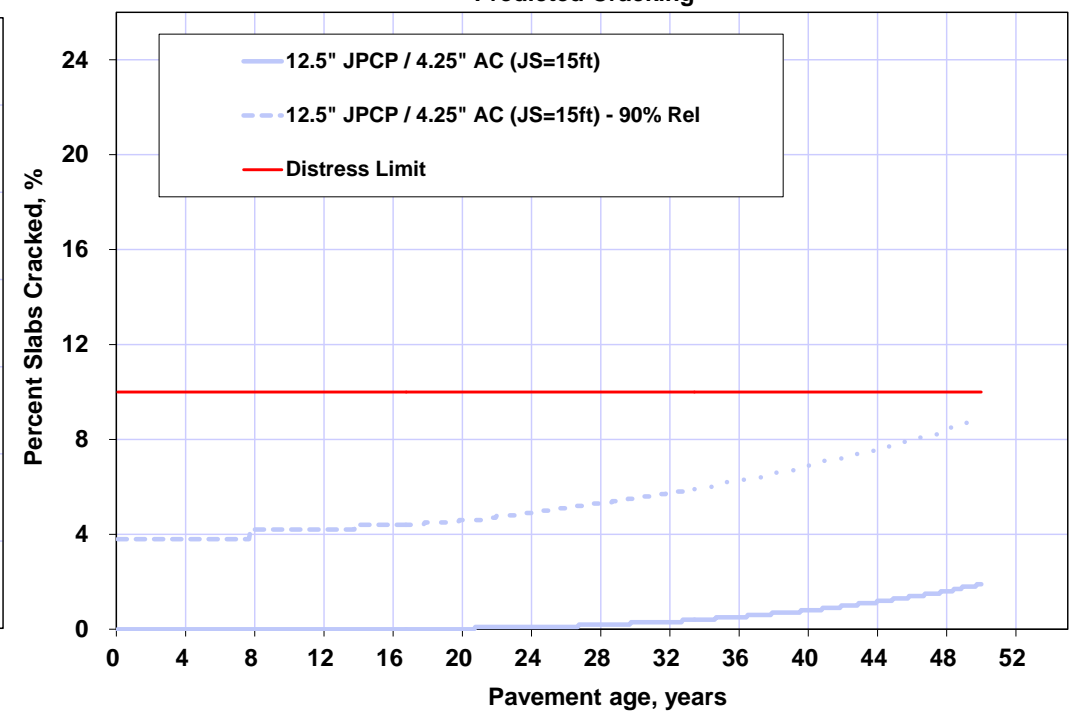
# MEPDG SHOWS NO STRUCTURAL REHABILITATION REQUIRED FOR 50+ YEARS

## The Pavement Design Criteria is 30-year Design

**Predicted Faulting**  
Predicted Faulting



**Predicted Cracking**  
Predicted Cracking



**Faulting and Cracking remain well below distress limits for 50 years**

# IRI INDICATES REHABILITATION NEEDED AT APPROXIMATELY YEAR 36

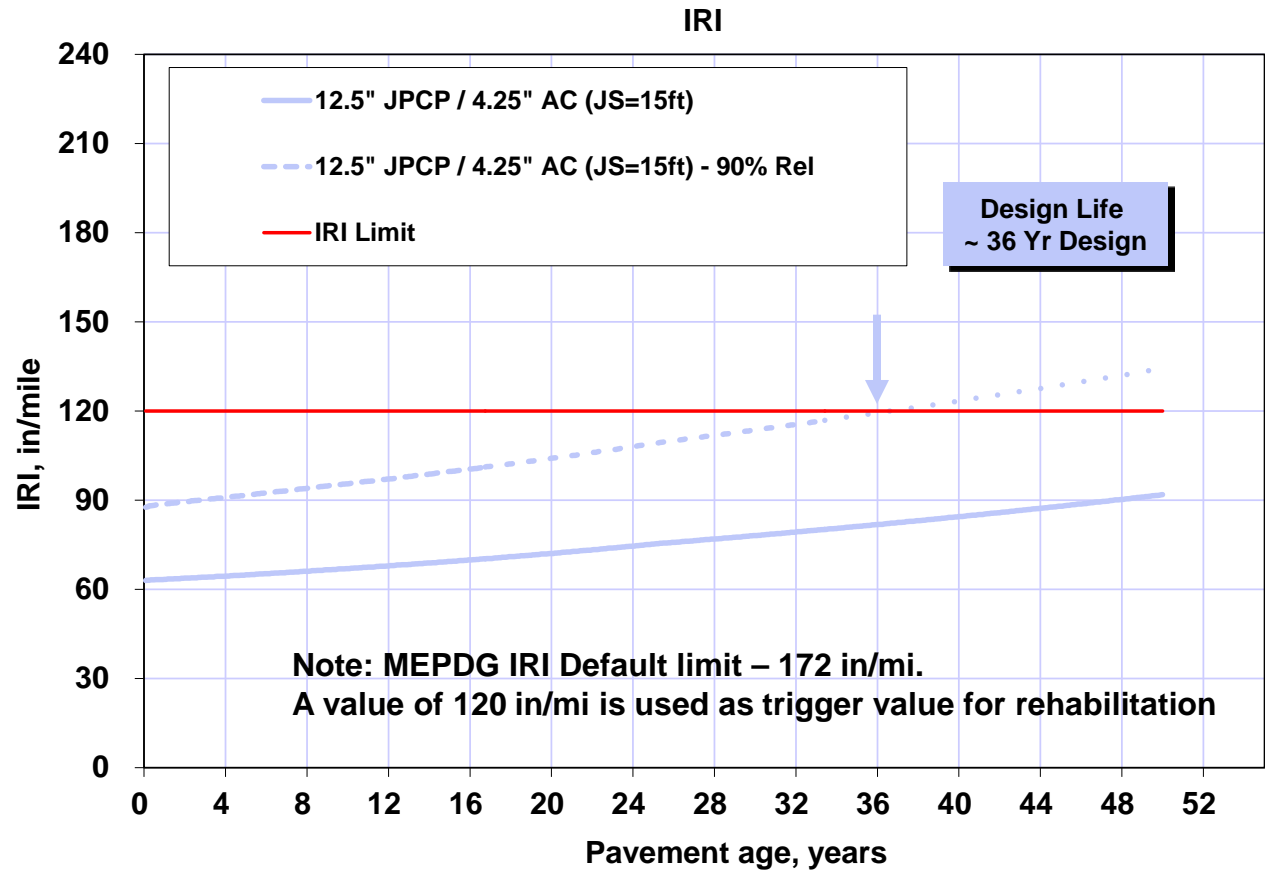
The Pavement Design Criteria is 30-year Design

## MEPDG Predicted Performance

While the design meets the performance requirements, there are opportunities to improve design

### Potential Improvement Considerations

- Iterate Thickness
  - 11", 10", 9", etc
- Removed Asphalt Base & replaced with 1" AC interlayer & 4" CTB
- Decreased joint spacing to 13 ft
  - Local Agg has high COTE
- Change shoulder type and decrease thickness



Arrows indicate year of predicted 1<sup>st</sup> rehabilitation for that given pavement

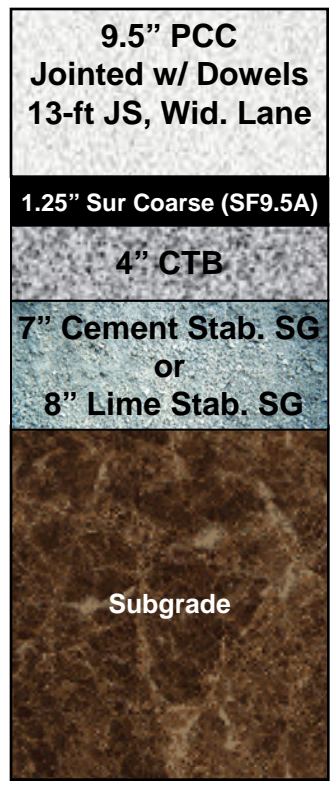
# MEPDG SHOWS OTHER CONCRETE SECTIONS MEET THE 30-YEAR DESIGN CRITERIA

Changing designs also changed the controlling distress

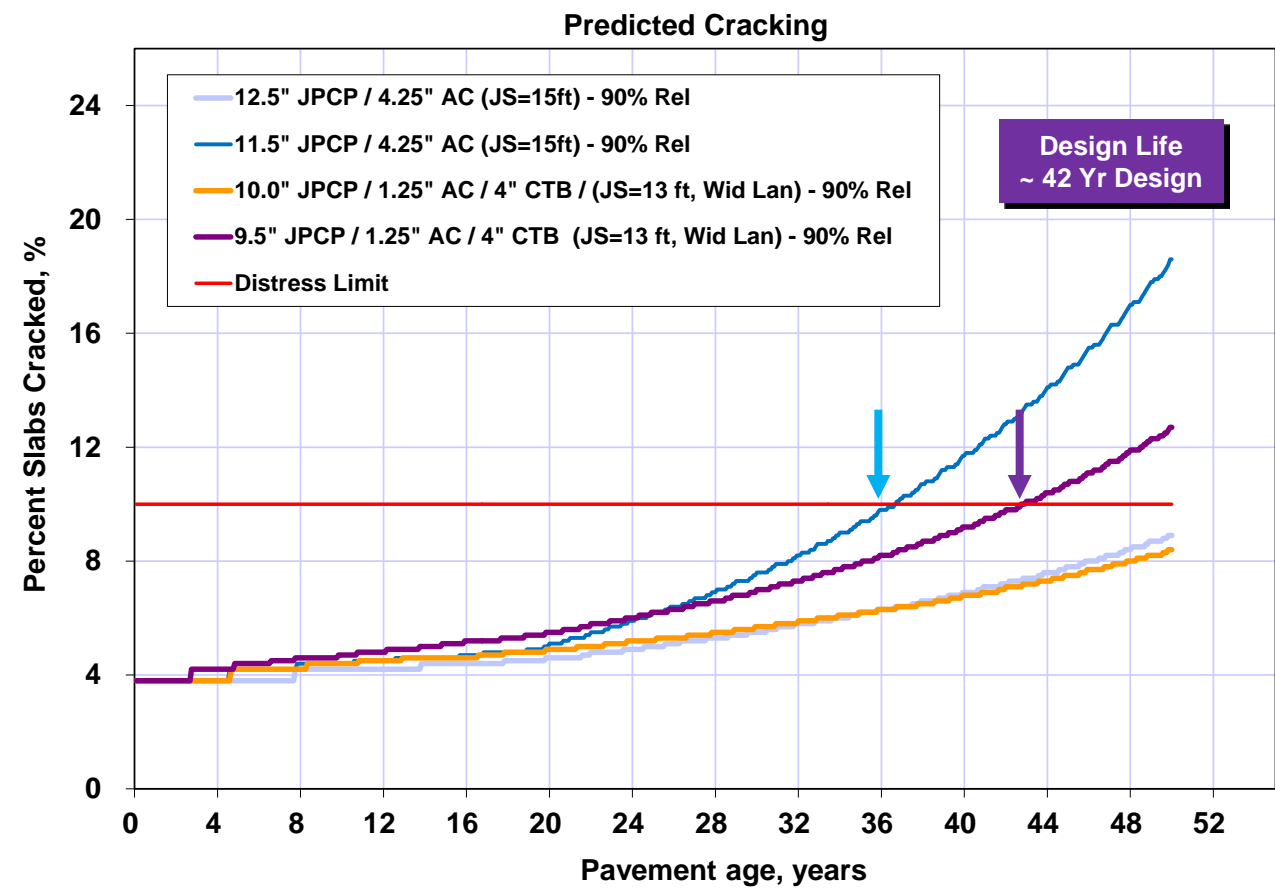
Standard Concrete



MEPDG Concrete



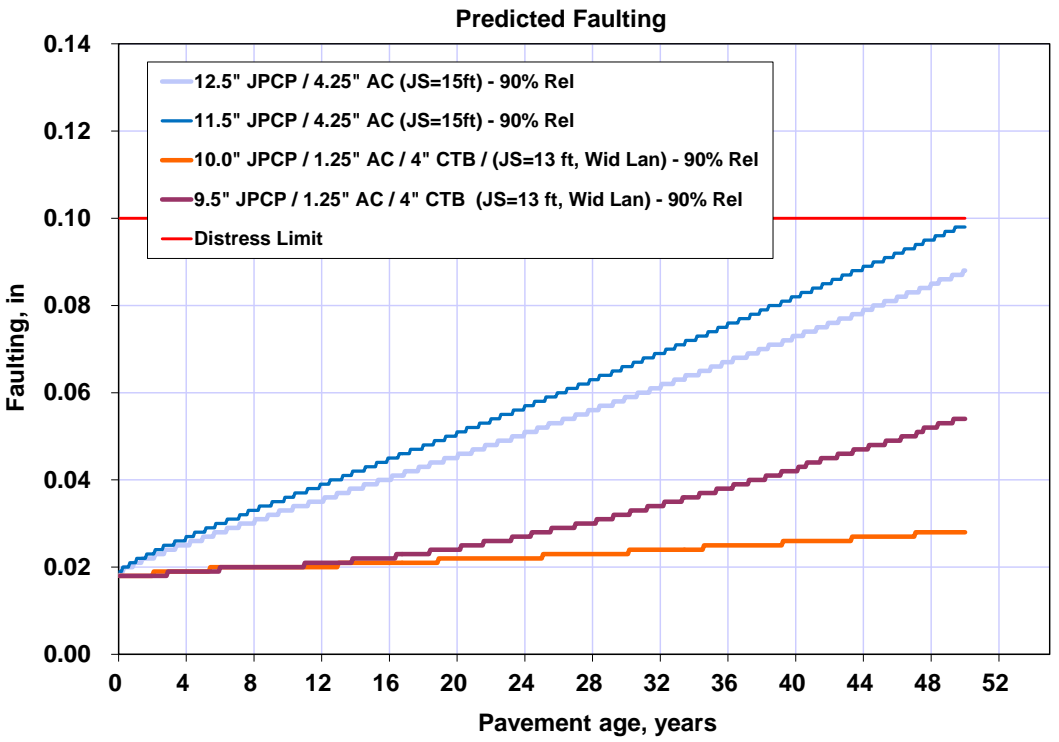
MEPDG Predicted Performance



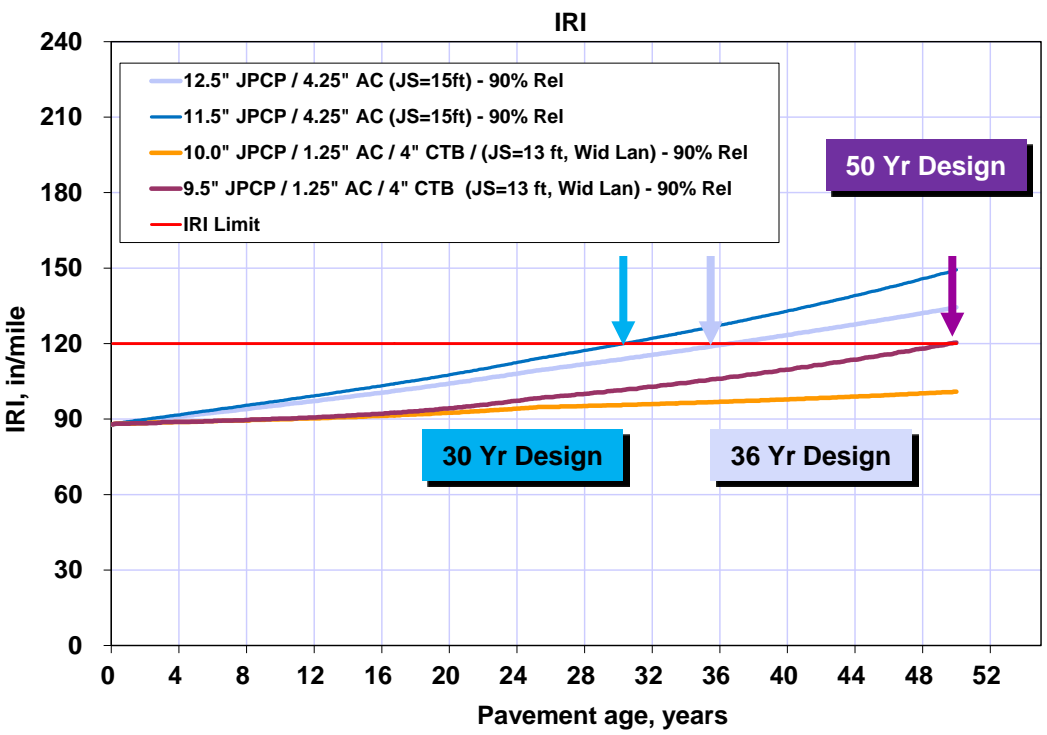
9.5" Jointed Pavement with Widened Lanes & 13-ft joint spacing is a 42-Year design

# OTHER DISTRESS SHOW THAT THE 9.5" JPCP WITH WIDENED LANES & 13-FT JOINT SPACING MEETS DESIGN CRITERIA

Predicted Faulting



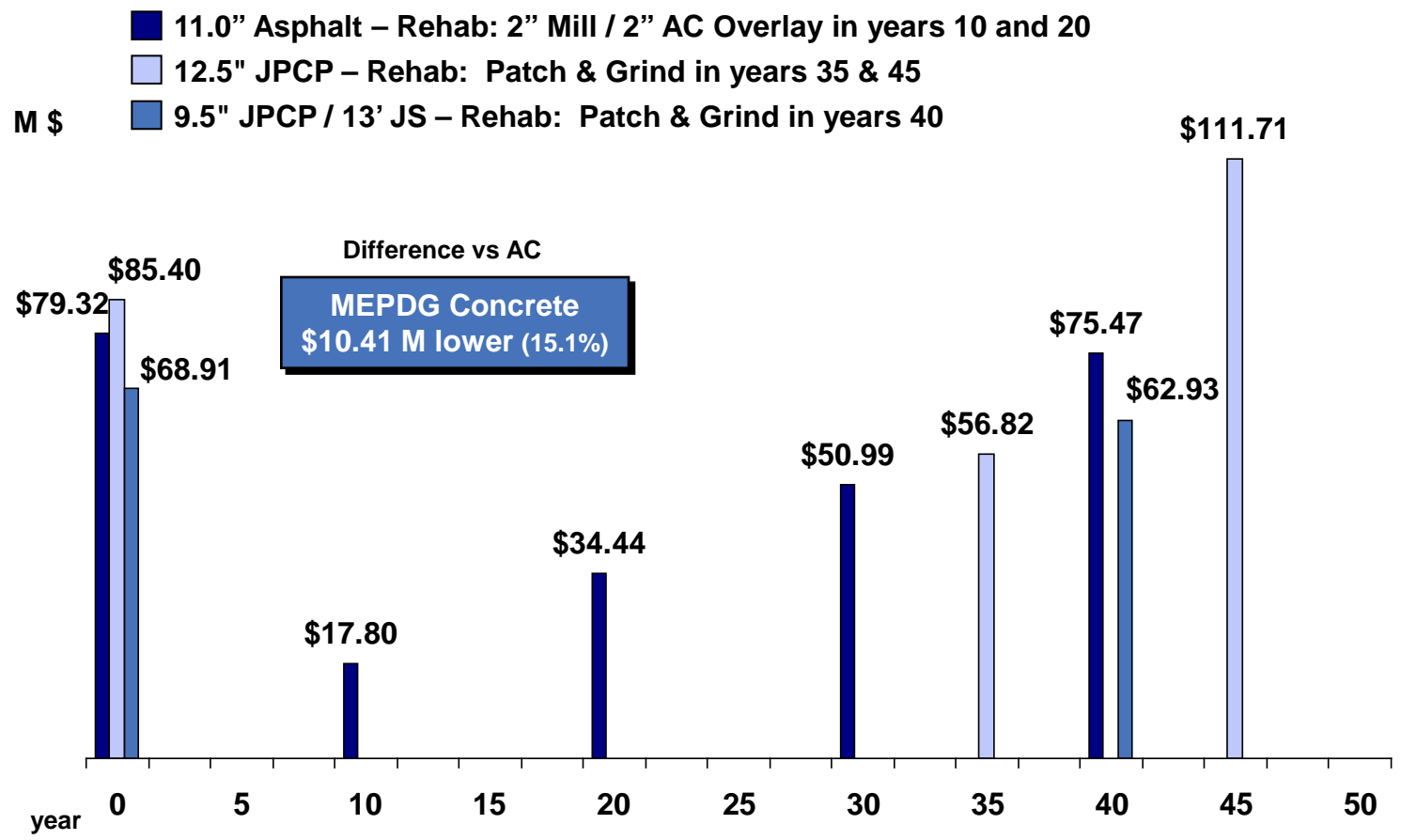
Predicted IRI



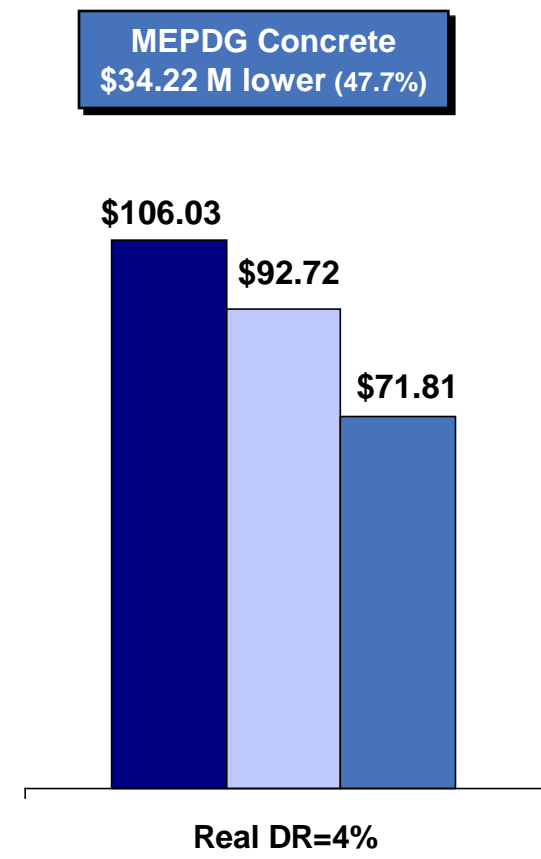
# THE OPTIMIZED PAVEMENT HAS BOTH LOWEST INITIAL COSTS & FUTURE REHABILITATION COSTS

## LCCA Procedures based on MEPDG/DARWIN-ME

Nominal Expenditures by Pavement Type (\$ M)



LCC Net Present Value (\$ M)



Costs for 21 miles, 3 lanes plus Shoulders. Initial Costs include Pavement, base, and subgrade stabilization materials and labor  
 Rehabilitation costs – AC Activities based on NCDOT Schedules with same activities continued throughout 50 year analysis  
 Concrete activities based on MEPDG (no salvage) – 3% Patch & 100% Grind in yr 35, 5% Patch & 100% Grind in yr 35

# IN DETERMINING FINAL PAVEMENT SELECTION

## Need to look at “Key Risk Factors” and “Stress the Results”

### Key Risk Factors

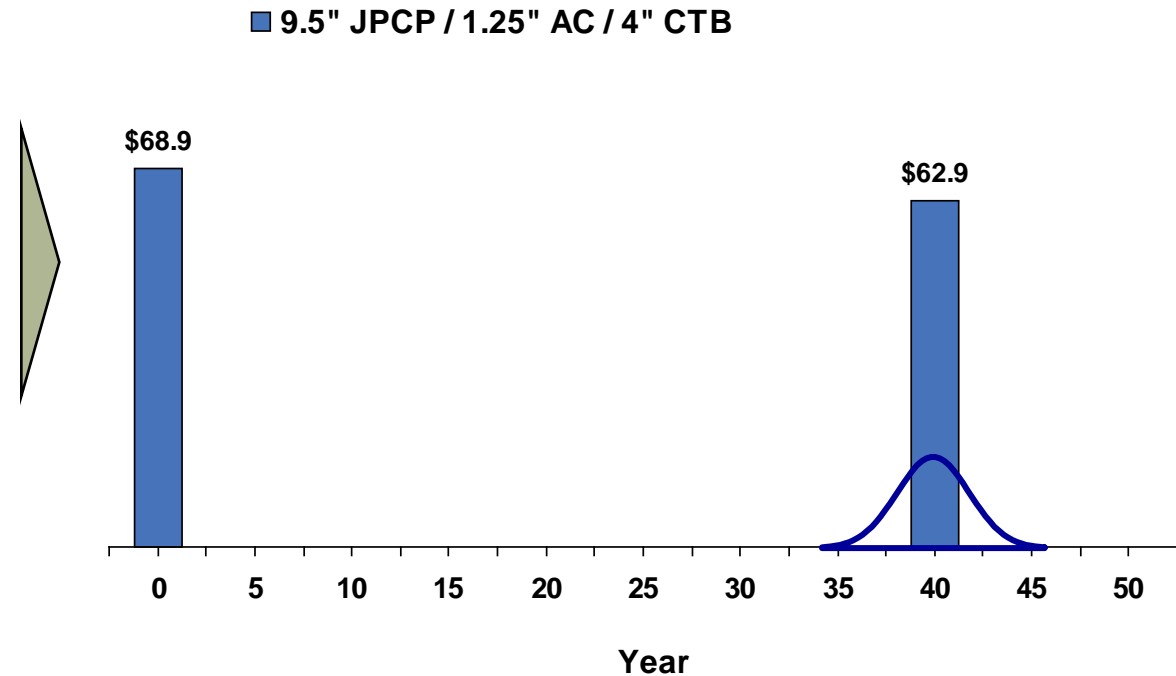
#### Performance

- How do changes in “when” each rehabilitation activity occurs affect results?
  - MEPDG is new. How do I know pavement will last as long as predicted?

#### Traffic

- How does different designs respond to changes in traffic
  - Increase (how does it affect rehab cycles?)

### Nominal expenditures by pavement type (\$ M)

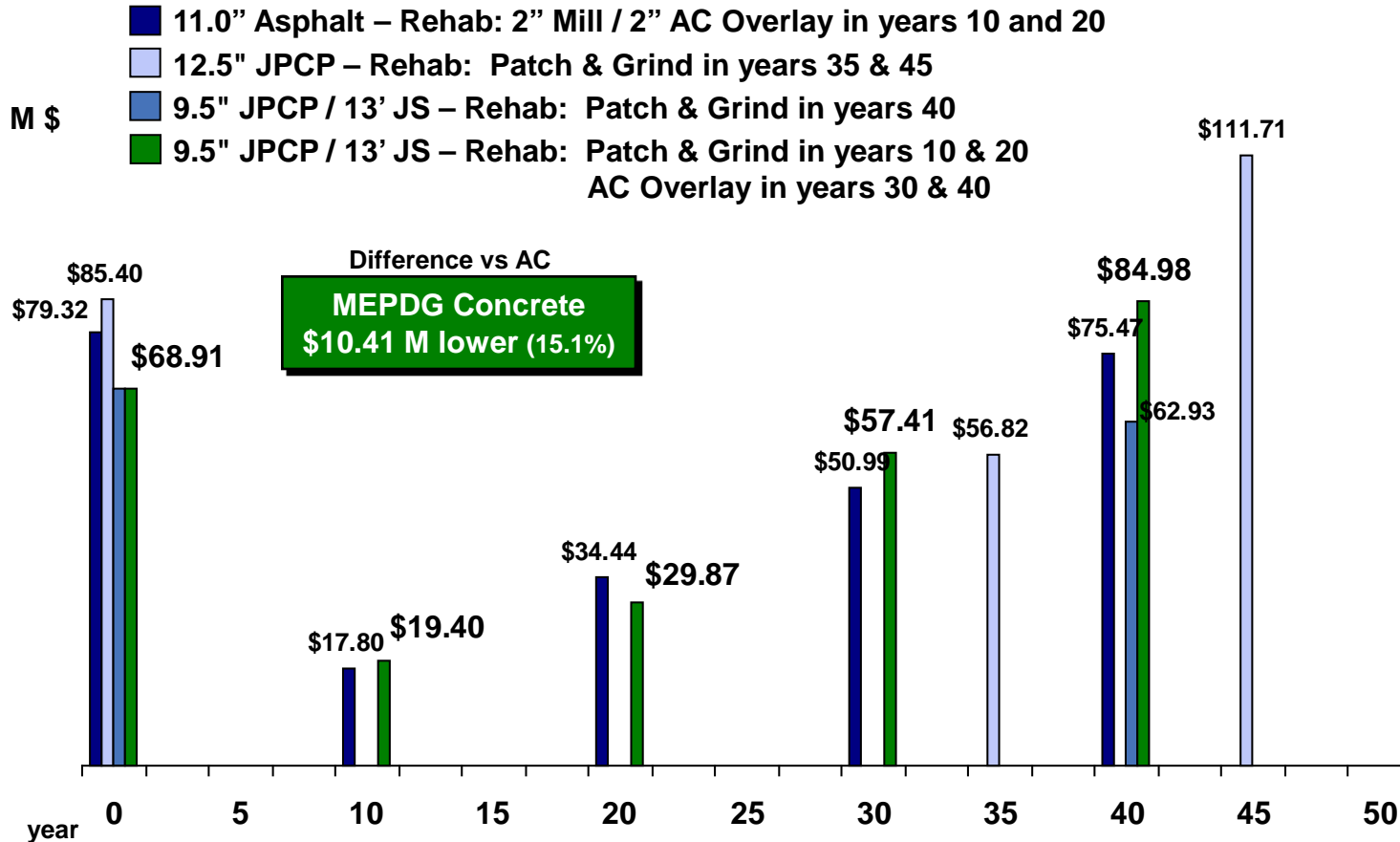


Life cycle cost analysis allows designer to consider variability with inputs and impact on performance

# FOR OPTIMIZED CONCRETE NOT TO BE A GOOD CHOICE, PAVEMENT REHAB MUST START IN YEAR 10

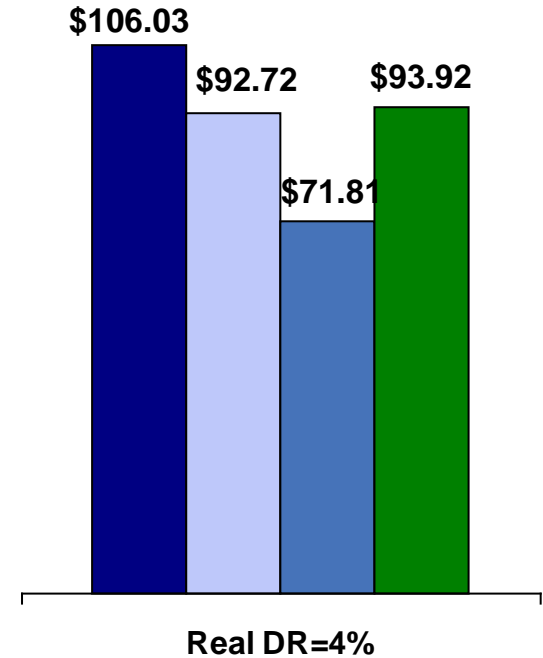
(eg. Pavement Rehabilitation occurs 30 years earlier than predicted)

Nominal Expenditures by Pavement Type (\$ M)



LCC Net Present Value (\$ M)

**MEPDG Concrete**  
Approx same as Original Concrete  
Still much lower than Original AC



Initial Cost Savings are much greater than the risk of additional rehabilitations due to pavement underperforming

# AGENDA

**My view on DARWin-ME / MEPDG**

**Pavement Design Procedure**

**Example – Monroe Bypass, Charlotte NC**

**Initial Costs Savings**

**Life Cycle Costs Savings**

**Stressing the Results**

**Other issues**



# SPECIFIC ISSUES / USES INVESTIGATED WITH MEPDG/DARWin-ME

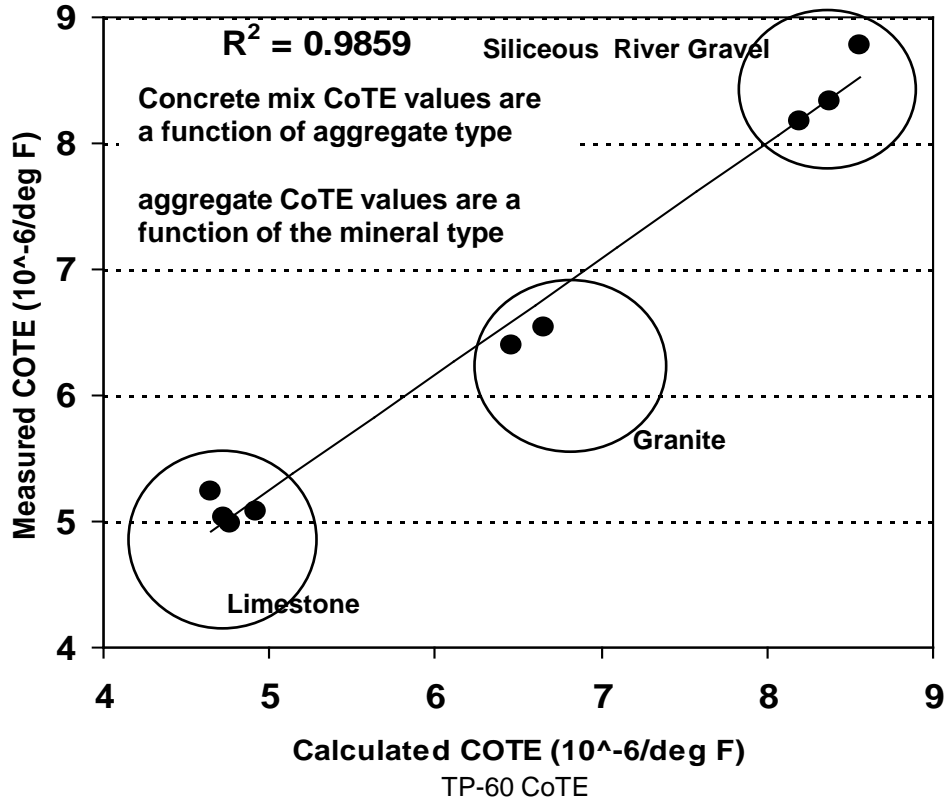
<b>Coefficient of Thermal Expansion (CoTE)</b>	<ul style="list-style-type: none"><li>• Sensitivity of Designs to CoTE</li><li>• AASHTO TP-60 vs. AASHTO T-336 Values<ul style="list-style-type: none"><li>• Systematic error or shift due to using the wrong calibration coefficients for the CoTE of the reference materials in the standard test procedure.</li></ul></li></ul>
<b>MEPDG IRI Results</b>	<ul style="list-style-type: none"><li>• There are designs where I could not get IRI to meet design criteria<ul style="list-style-type: none"><li>• Can not look at distress items as single, un-related items</li></ul></li></ul>
<b>Monthly Impacts</b>	<ul style="list-style-type: none"><li>• Month of Construction is an input, but is unknown at the time of paving<ul style="list-style-type: none"><li>• How big is this?</li></ul></li></ul>
<b>Improving Estimate of Rehabilitation Timing</b>	<ul style="list-style-type: none"><li>• Rehab occurs when Reliability line hits pre-defined distress limits</li><li>• Additional rehab timing can also be determined this way</li></ul>

**Project Sensitivity Analysis can help address risk of pavement underperforming**

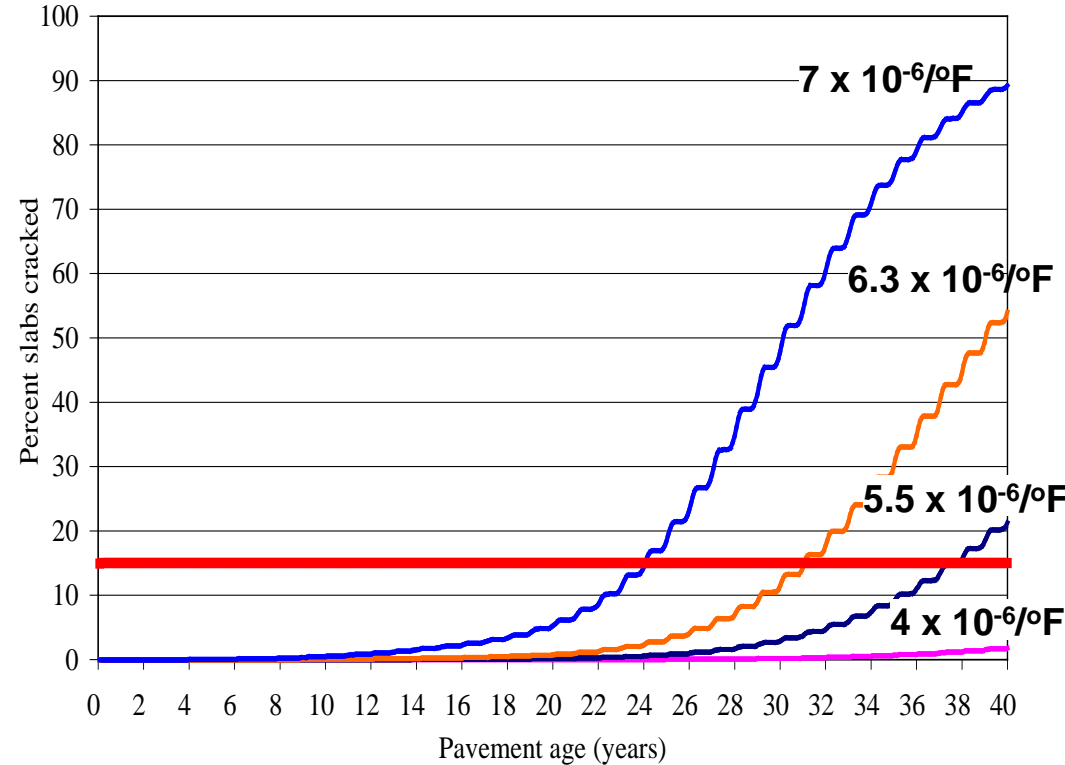
# COTE IMPACTS JOINTED PAVEMENT CRACKING

## Higher COTE Values increase cracking

CoTE Variations



CoTE Impacts on JPCP Cracking

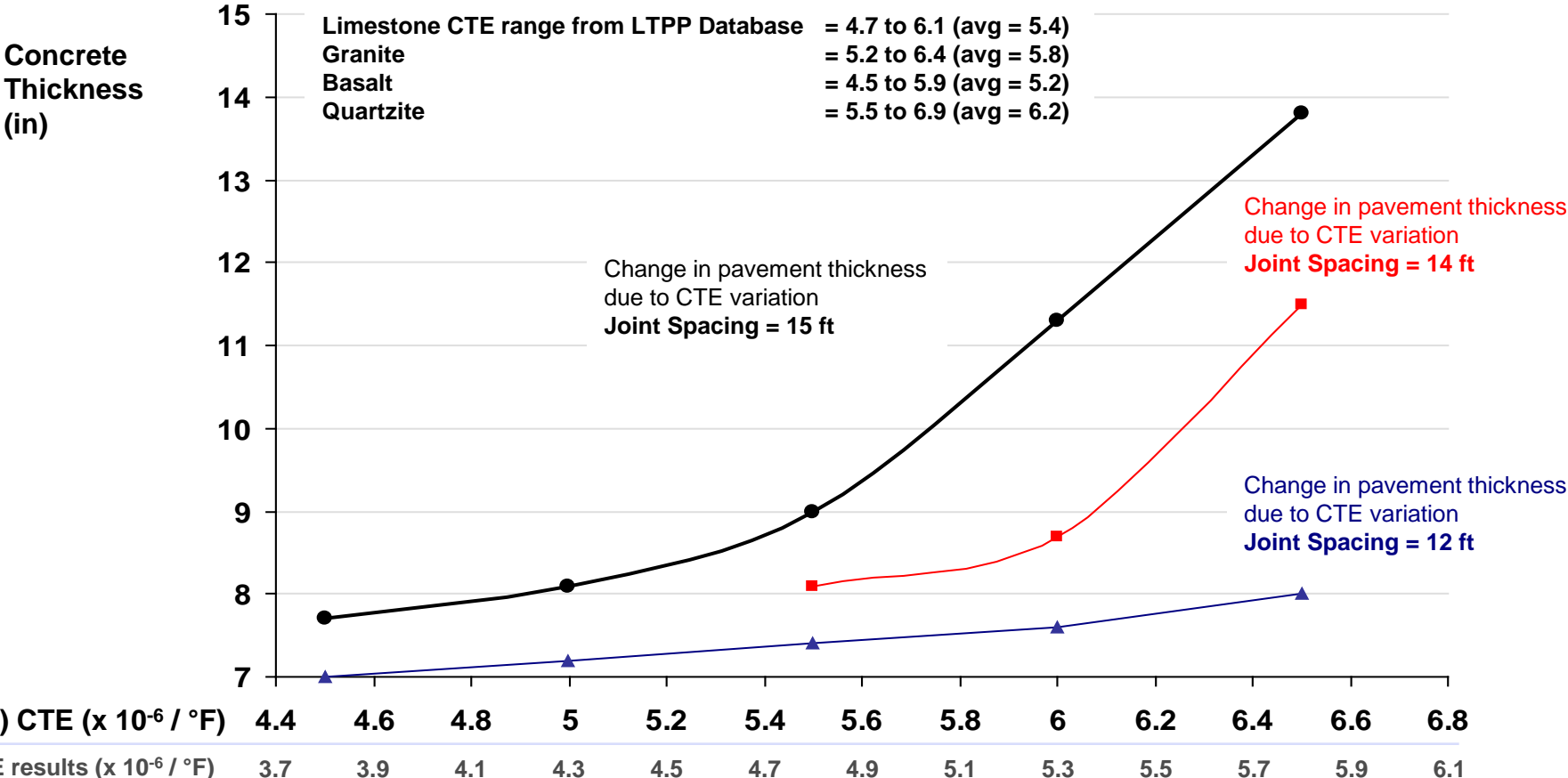


Cracking is due to the curling of the slab so the key design aspect is to be able to deal with the curling



# CURLING EFFECT DUE TO CoTE CAN BE MITIGATED WITH THICKNESS OR JOINT SPACING

Shorter Joints decrease the moment arm for slab uplift



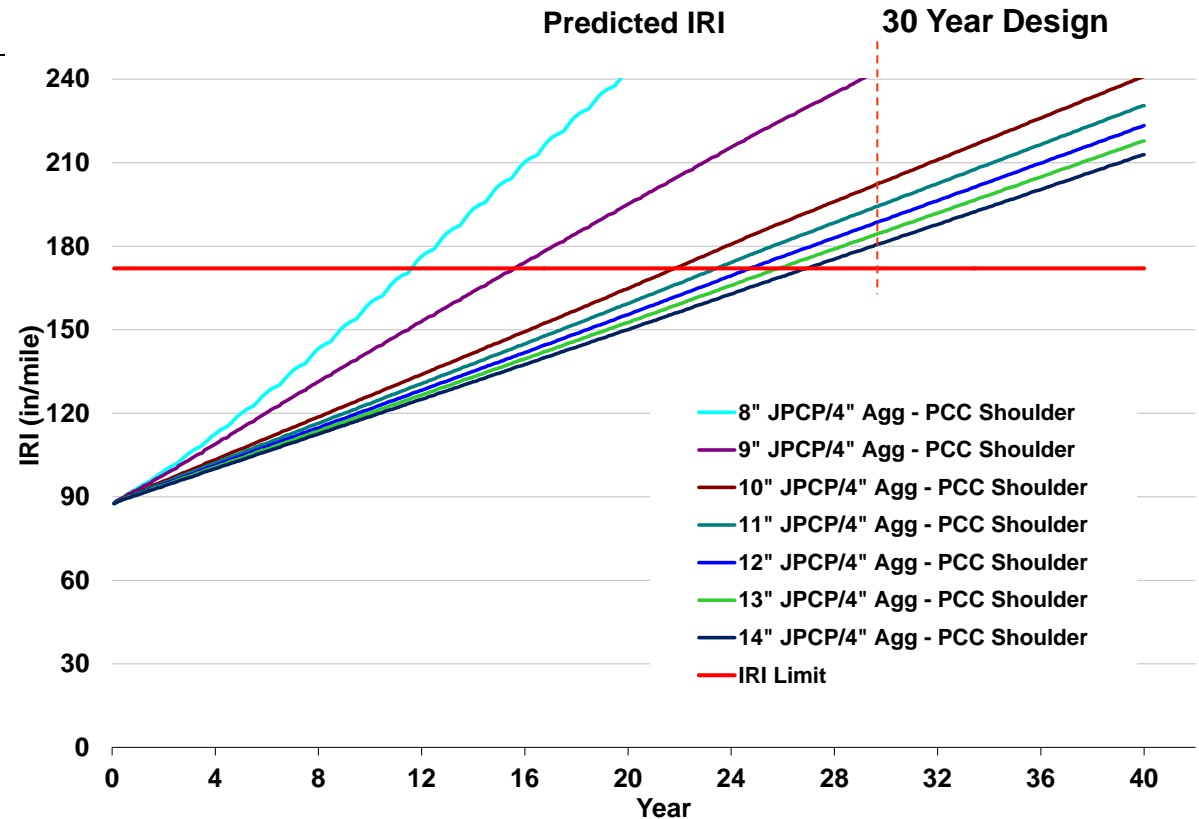
**Issue is not how low CoTE is, but how to design pavements based on the CoTE value for your aggregates**

- To convert from AASHTO T-336 CTE results to values that can be used MEPDG (TP-60), add 0.695 to AASHTO T336 results
- Pavement = 15 ft jointed concrete pavement on 4" AC base on 12" limerock base. Subgrade = A-3 Soil. 20 Year Design with 2-Way AADTT = 15,000, 2 lanes in each direction, Climate = Orlando FL

# FOR SOME GIVEN SOIL AND CLIMATIC CONDITIONS IRI DESIGN CRITERIA CAN NOT BE MET ...

## Rochester MN Example

- 30 year Design life (40 year MEPDG Analysis)
- 2-way AADTT (Truck Traffic) = 3,100
  - Single Lane
  - Estimated ESALs = 52.17 M
- Design Features
  - Thickness - varies
  - 4" Crushed Stone (Granular) Base
  - 15-ft Joint Spacing
    - 1.5" Dowels for 10" or greater
    - 1.25" Dowels for less than 10"
  - Tied Concrete Shoulder
  - Limestone Aggregate
    - CoTE =  $5.5 \times 10^{-6} / ^\circ\text{F}$
  - CH Suggrade (High Plastic Clay)

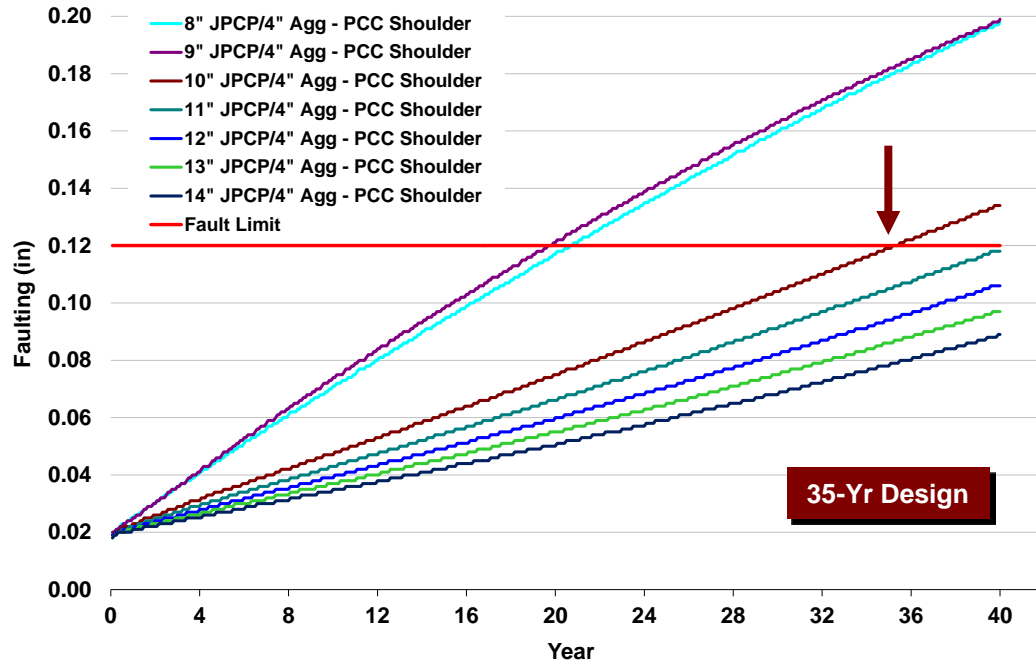


No matter the concrete thickness, the 30 year IRI default criteria can not be met

# ... BUT MEPDG SHOWS CRACKING AND FAULTING LIMITS CAN BE MET WITH A 10-IN PAVEMENT ...

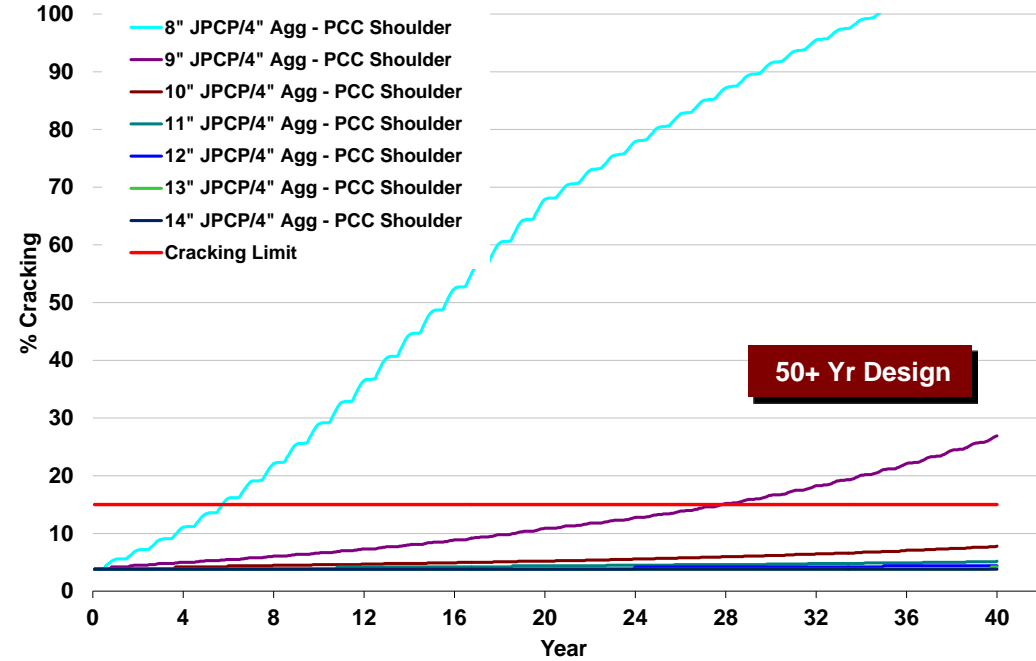
**Predicted Faulting**

Predicted Faulting



**Predicted Cracking**

Predicted Cracking



**Question: How does a 14" Pavement IRI increase if there is no faulting or cracking?**

## ... FOR THESE CONDITIONS USE THE LOWEST CONCRETE THICKNESS FROM CRACKING & FAULTING

<b>Model</b>	<ul style="list-style-type: none"> <li>• <math>IRI = C1*(Crack) + C2*(Spall) + C3*(Fault) + C4*(Site\ Factor)</math></li> </ul>
<b>Site Factors</b>	<ul style="list-style-type: none"> <li>• Freezing index</li> <li>• Percentage of subgrade material passing the 0.075-mm sieve.             <ul style="list-style-type: none"> <li>• Relates to the potential for soil movements due to frost heaving and settlement</li> </ul> </li> </ul>
<b>Discussion</b>	<ul style="list-style-type: none"> <li>• When fault and cracking are low, IRI distress level is being controlled by “Site Factors”</li> <li>• Site factors can not be altered by changing pavement designs</li> </ul>

**Adding additional thickness to control IRI is costly and not warranted**

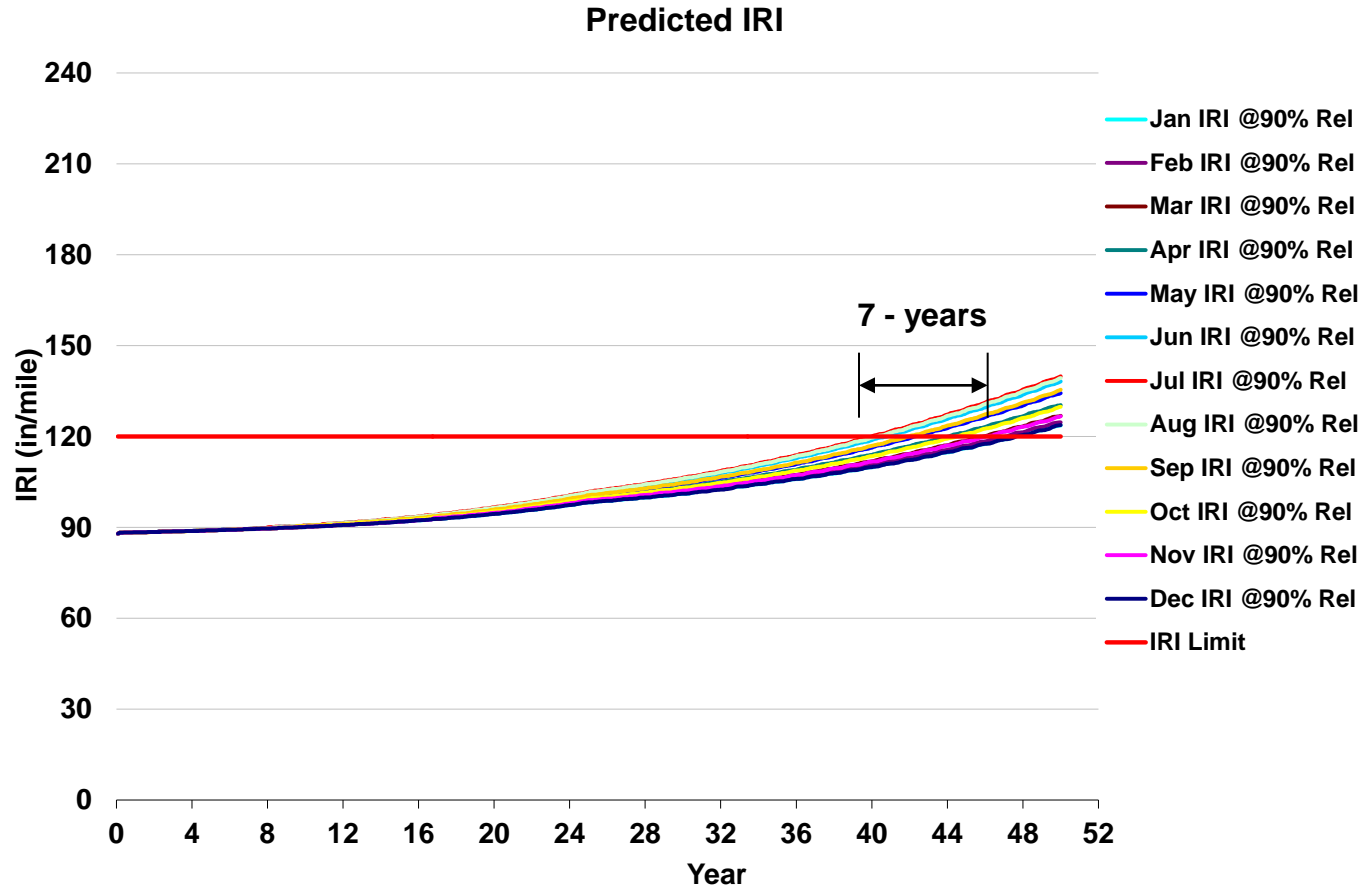
# CONSTRUCTION MONTH HAS SOME IMPACT ON MEPDG RESULTS

## IRI Results

### Construction Month Impacts the PCC Zero-Stress Gradient

Eg - Texas Example

- January – PCC Zero-Stress = 65
- February = 70
- March = 79
- April = 91
- May = 101
- June = 107
- July = 112
- August = 112
- September = 106
- October = 92
- November = 80
- December = 80

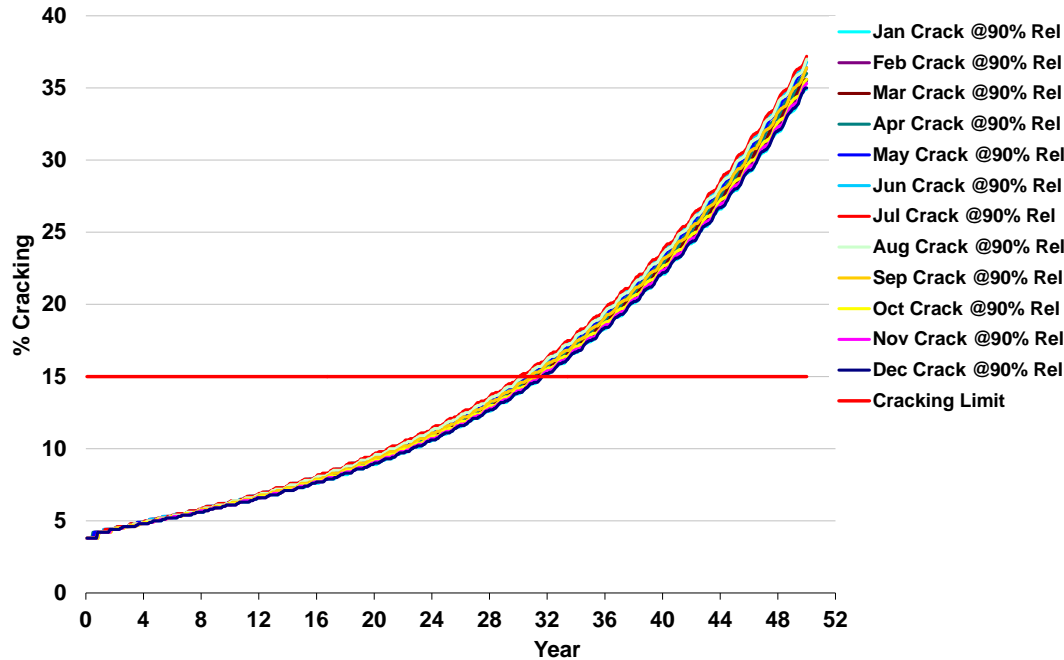


I design pavements using the highest PCC Zero Stress Month (usually July or August)

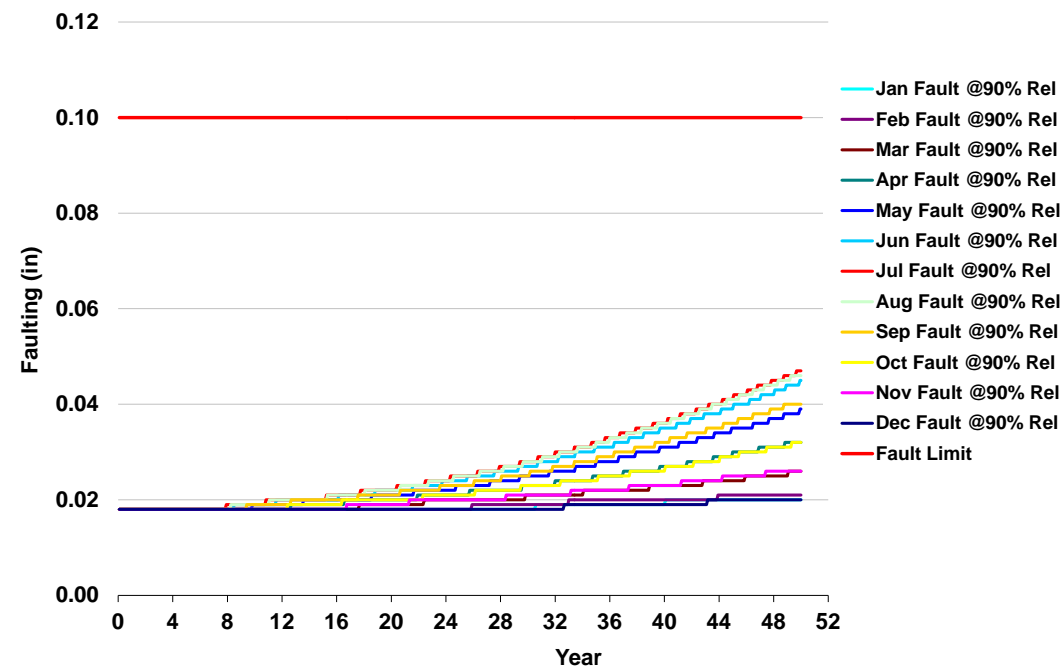
# CONSTRUCTION MONTH IMPACT ON MEPDG RESULTS FOR CRACKING & FAULTING

For this particular case. Controlling distress can change

Predicted Cracking



Predicted Faulting



The highest PCC Zero Stress Month typically shows the worst performance



# MEPDG PERFORMANCE CURVES SHOW WHEN TO DO REHABILITATION ACTIVITIES

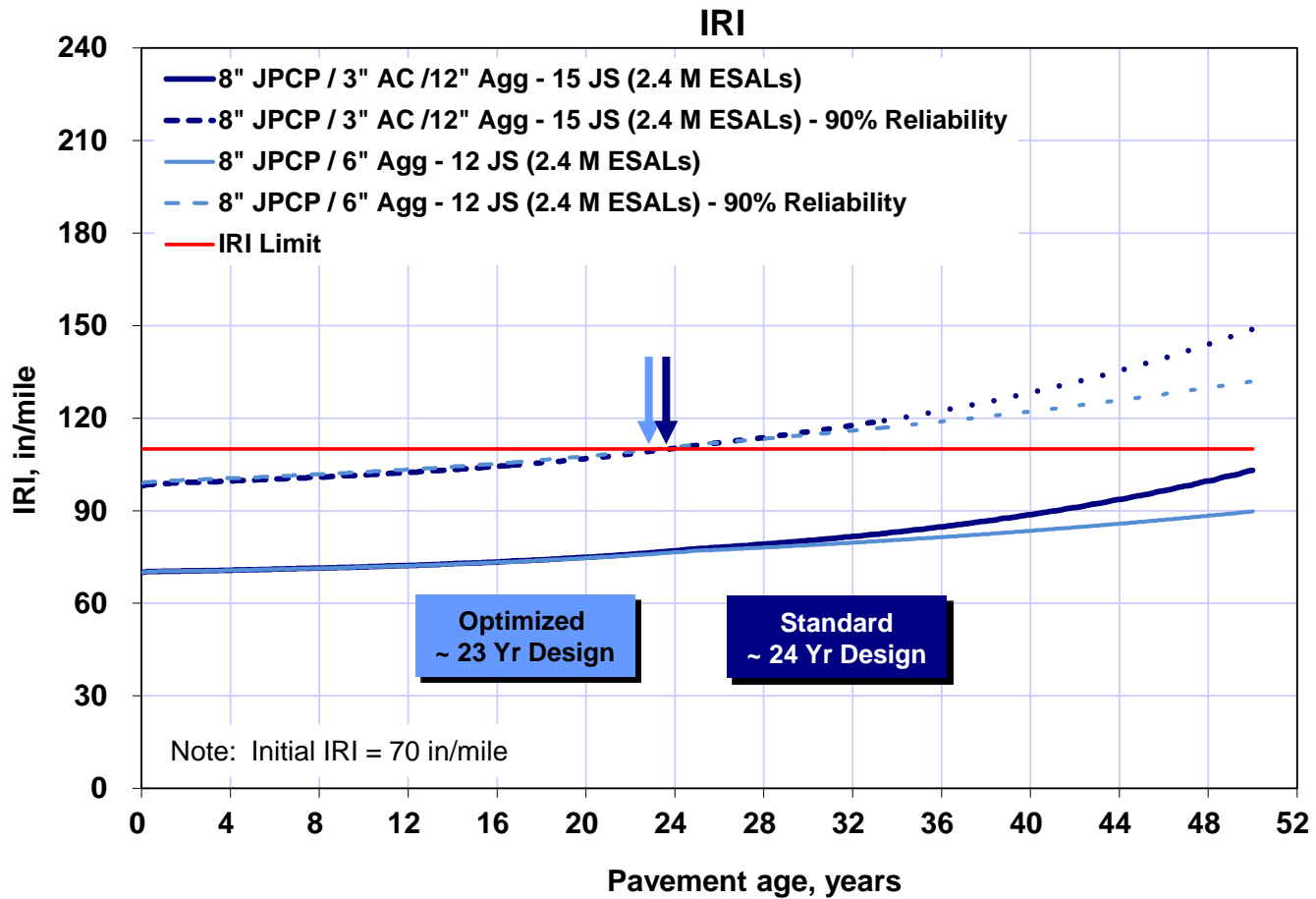
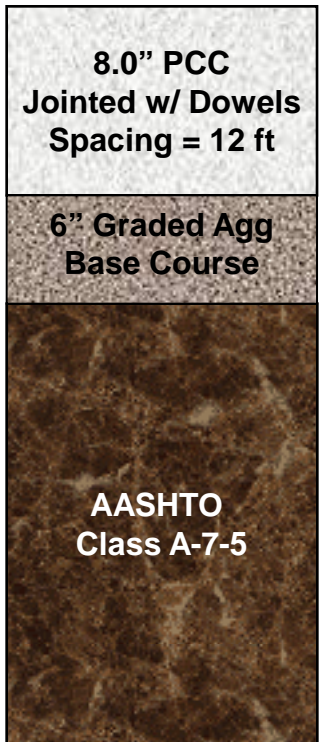
Performance criteria is 20-year design

MEPDG Predicted Performance

## Standard Design



## Optimized Design



Optimized Pavement has same predicted IRI as the Standard Design

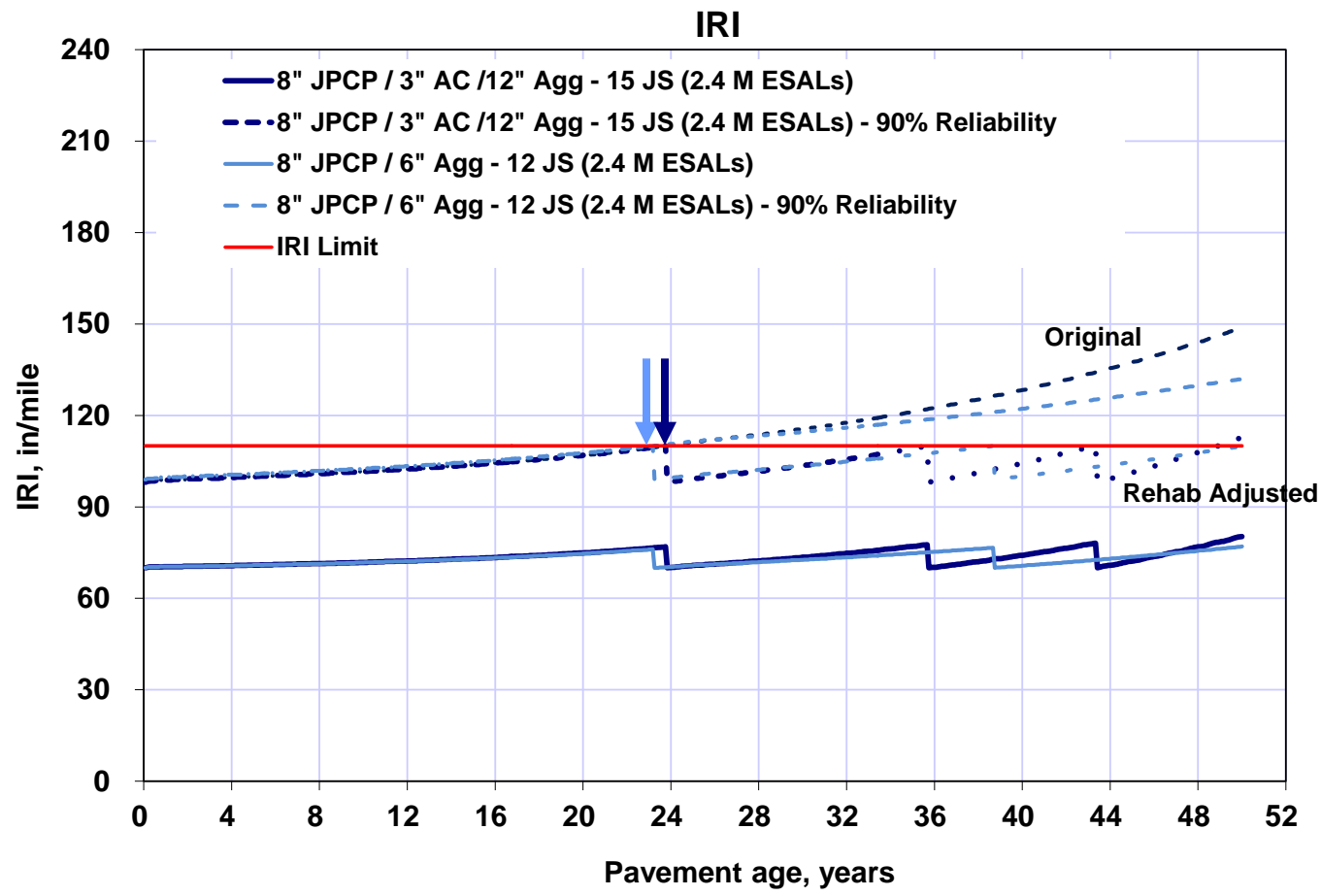
# ADJUSTED PERFORMANCE CURVES TAKING INTO ACCOUNT REHABILITATION TIMING

## Rehab Schedules

**Original 8" JPCP / 3" AC / 12" Agg**  
 CPP 1 = 24 Yrs  
 CPP 2 = 36 Yrs  
 CPP 3 = 45 Yrs  
 ACOL = 50 Yrs

**Optimized 8" JPCP / 6" Agg - 12' JS**  
 CPP 1 = 23 Yrs  
 CPP 2 = 38 Yrs  
 CPP 3 = 50 Yrs

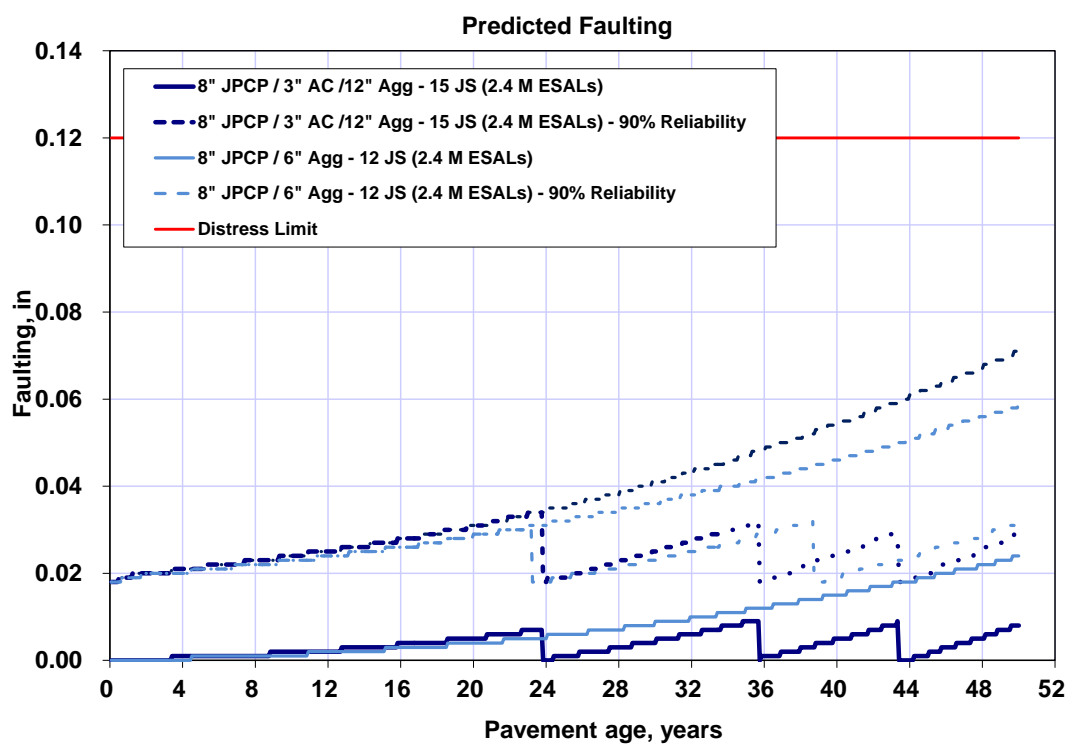
## MEPDG Predicted Performance with Rehabs included



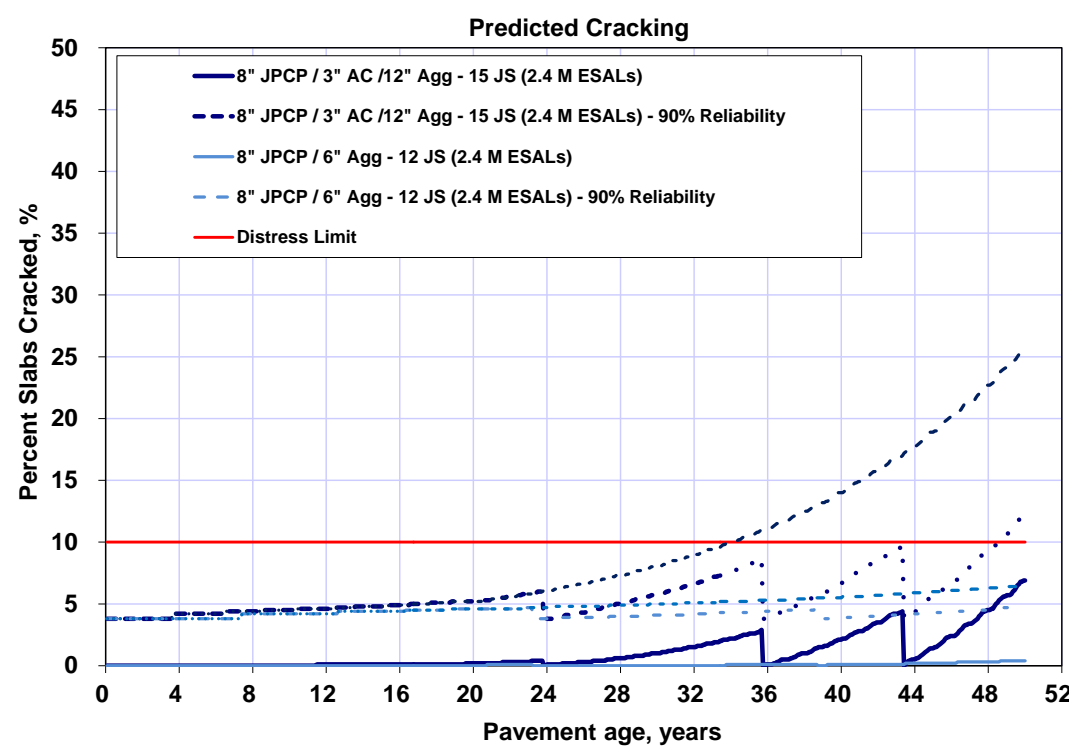
Time between rehabilitation changes with designs and decreases due to increasing traffic

# OTHER DISTRESS ALSO SHOW THAT THE 8" JPCP WITH SHORT JOINT SPACING PERFORMS BETTER

Predicted Faulting



Predicted Cracking



# SUMMARY

- 1 **DARWin-ME / MEPDG can be used to lower concrete pavement's initial cost**
  - They will still have good long term performance
  - They still have “Low Cost of Ownership” and Low Life Cycle Cost
  
- 2 **Designs should be developed using both Engineering & Economic Analysis balancing the initial costs and long term performance**
  - **Engineering**
    - Determine concrete thickness and match rehabilitation schedules to the design using DARwin-ME / MEPDG predictions
  - **Economic**
    - Estimate initial and LCCA costs for each design
  
- 3 **Adjust structures to meet the design, performance and cost requirements**
  - Optimizing concrete pavements is more than just cutting thickness
  - Other “features” have a significant impact on performance & cost
    - Each design feature is a balance between performance and cost