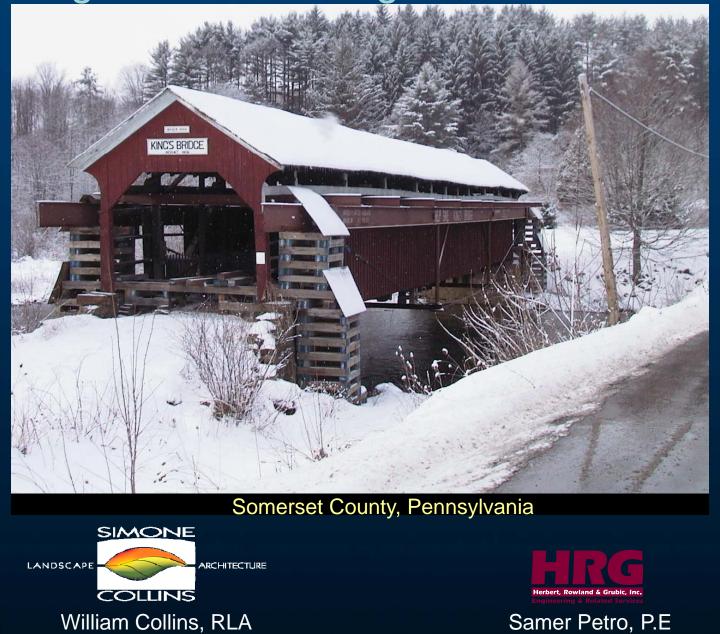
Kings Covered Bridge Rehabilitation



Samer Petro, P.E Kings Covered Bridge Rehabilitation



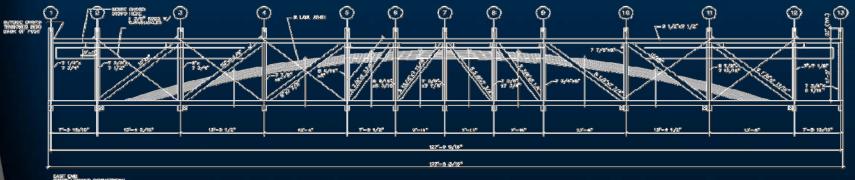
History

"Built" date on bridge (incorrect) probable construction period "Rebuilt" date on bridge - likely bypassed with steel highway bridge used as a King Family farm building PA funding for stabilization stabilization in place Saved by S. Alleghenies Conservancy Acquired – Middlecreek Township Completed – bridge and site

Superstructure

<u>Note</u> bay spacing and repair rods





EAST DAD INSIDE LOOKING DOWNSTREAM ELEVATION OF THE TRUSS

Multiple Kingpost Truss w/ Retrofitted Tied Arch

Second National Covered Bridge Conference, Dayton Ohio 2013.

Multiple Kingpost Truss with arch added

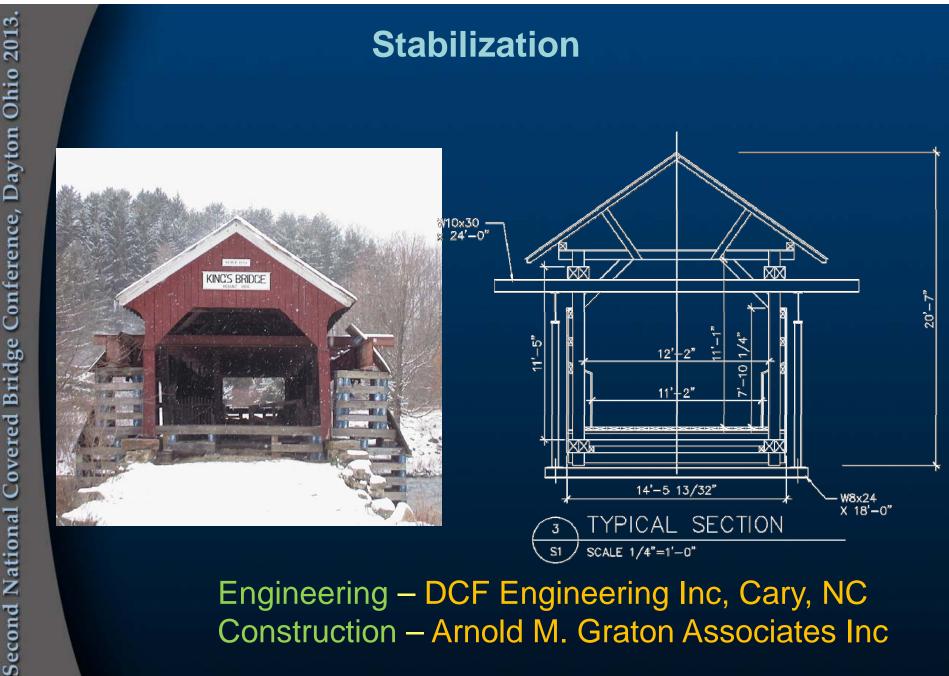




- Retrofitted 1906 with nail-laminated arches creating "Arch-Truss"
 - Arches "tied" to bottom chord

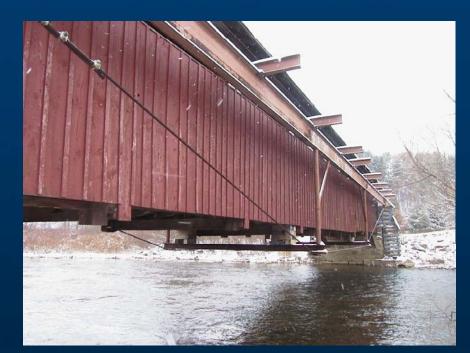
Bridge data

Length: 114 feet clear span Width: 14'-6" out to out Multiple Kingpost with Superstructure form: retrofitted arches (1906) **Primary Species:** White Oak hewn, circular, up & down **Timbers members: Substructure:** local limestone Hardware: round nails

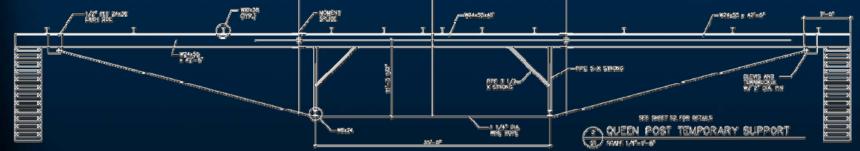


Engineering – DCF Engineering Inc, Cary, NC Construction – Arnold M. Graton Associates Inc

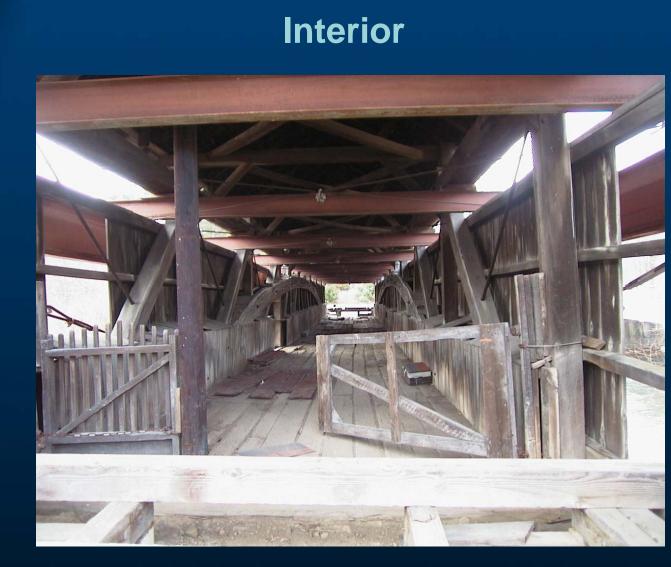
Temporary Superstructure



EAST IND INSIDE LODKING DOWNSTREAM

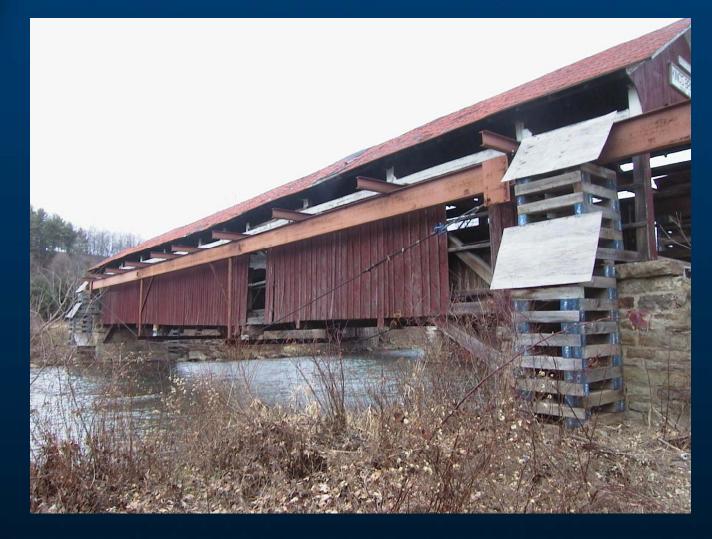


Queenpost-tensioned trusses w/ needle beams



Used as a barn after bypass

Failures

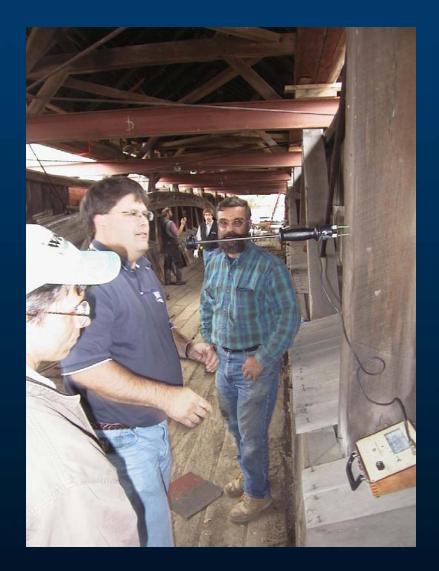


Both lower chords, some posts, some tie beams

Non-destructive Field Testing

Moisture testing

<u>Technical Assistance</u> USDA Forest Service *Morgantown, WV* Forest Products Lab, *Madison, WI*



Non-destructive Field Investigation / Testing





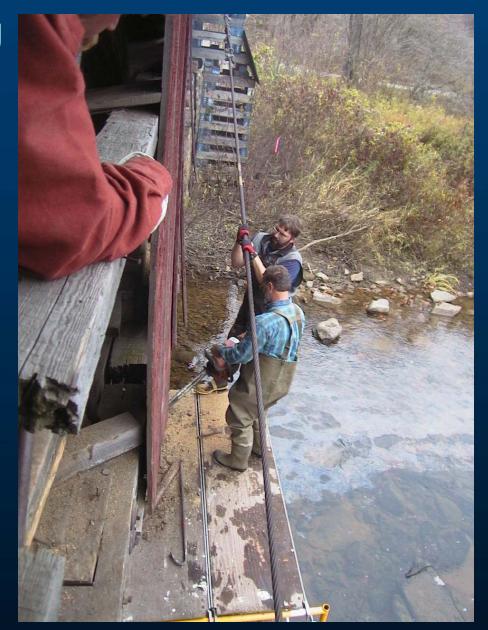
Resistance Drilling

Stress Wave Timer

Both chords had relative drilling resistance below 15%

Destructive Testing

4-foot lower chordsegment removed forspecies analysis byForest ProductsLaboratory



Mapping

Lower chord segments



Lower tie beam / tail of truss Post Failure from compression force of truss brace





Lower Chord Location of failure

Substructure



Horizontal thrust from broken chords, struts ceased to work

Bed timbers Note deformation

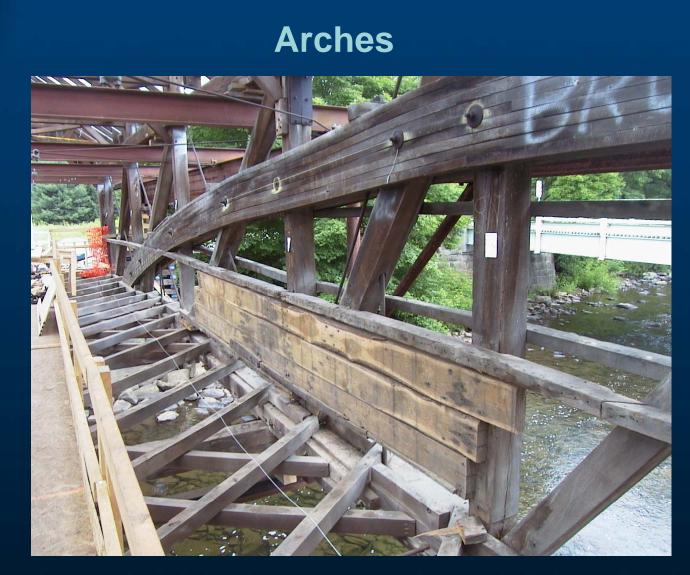


Retrofitted Struts Typical failure at seats





Nail-laminated retrofits



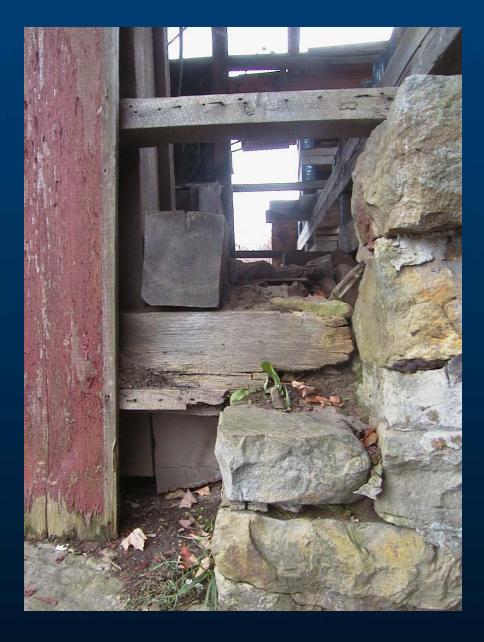
Deformation and previous interventions

Thrust Block at Portal Post



Bearing

Chord – abutment seat



Roof assembly

Mortised. pinned. and wedged horizontal X-bracing

Post - upper tie beam, rafter sill Cantilevered tie beams failed where leaks occurred

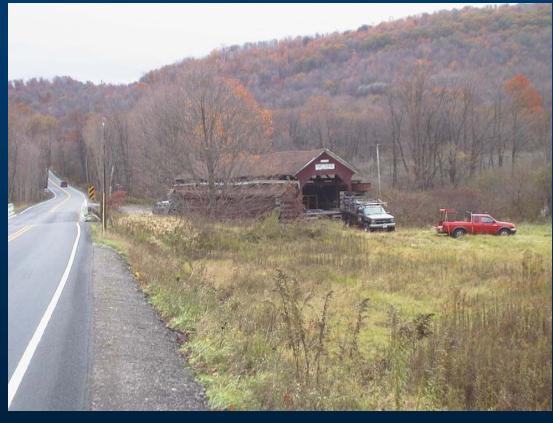


Truss Bracing water damage at truss post



Preservation Philosophy

Resource eligible for National Historic Register



Rehabilitate for pedestrian use Retain historic fabric as much as possible Replace with in-kind materials Meet the Secretary's standards

Structural Analysis



3-D rendering (STAAD Model) – Samer Petro, P.E.

- 3-D Model STAAD
- Provides member forces, moments, and deflections

Goal of Analysis:

- 1. Compare stresses to allowable stresses based on (NDS)
- 2. Understand arch-truss system



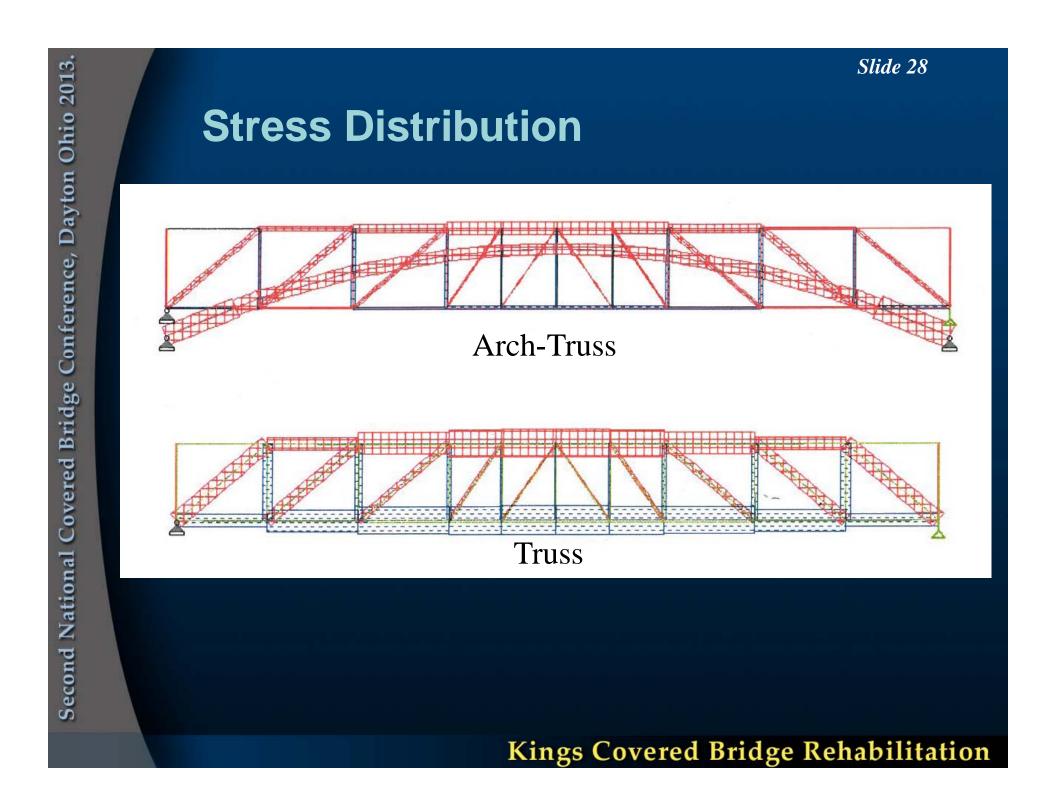
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- Ends of diagonals and ends of posts pinned (free to rotate)
 - Chord members and arches continuous
 - Multiple king post modeled as pinned

Loads

- **Dead loads:** approximated by field measurements and using white oak density of 43 pcf (MOE 1.4M psi)
- Live load: Pedestrian load of 85 psf (AASHTO)
- Snow Load: 35 psf
- Wind Load: 12.5 psf based on wind velocity on 100 mph (ASCE 7)
- Load Combination: [dead load + (wind+live+snow)*0.75] (ASCE 7)



Repair / Solutions



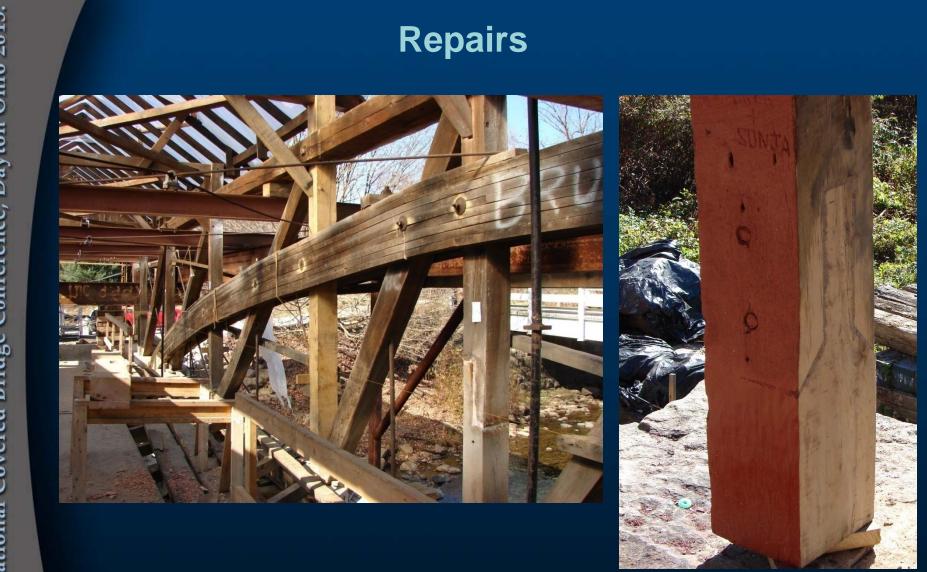


Dapped lower chords to permit laminated arches to extend forces into the abutment

Abutment Repairs



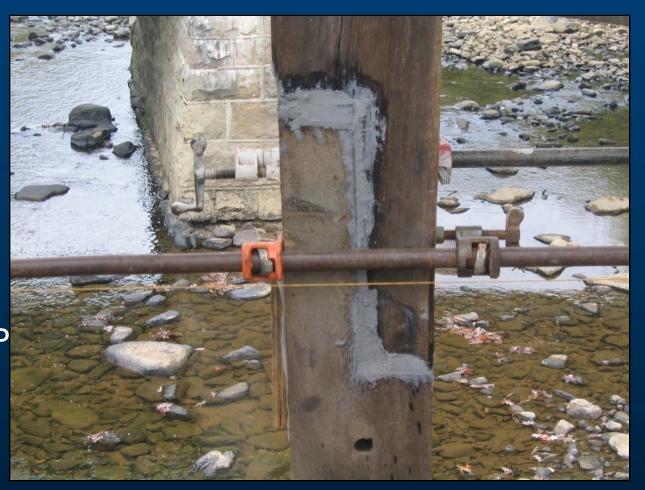
Excavate abutments, buttress with flowable backfill



Post splices and replacement

Repairs

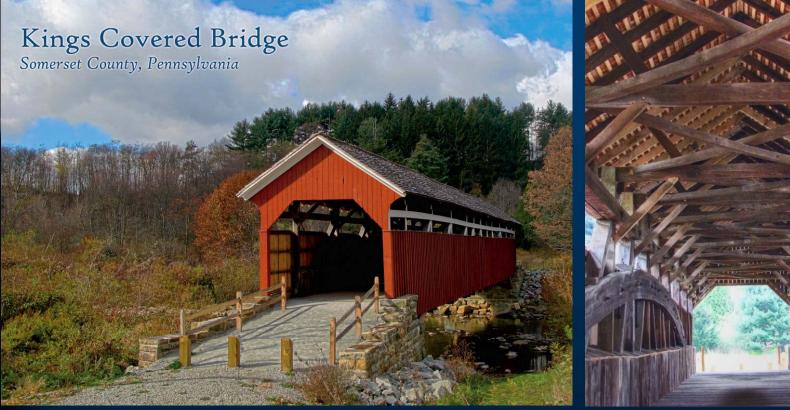
Post splice Innovative GFRP rods embedded in epoxy



Traditional Chord "joggle" white oak



Engineering Results



- Arch structurally dominant
- Trusses: lower chord in tension and max @ mid-span (expected)
- Arch-truss system: maximum compressive forces in arches at ends
- All member stresses well below allowable values
- Deflections due to dead and live loads below AASHTO limits

Completion



Before – stabilized

After – restored

First Annual Modjeski Award for Historic Preservation Preservation Pennsylvania



Completion



	Budget	
\$ 90,000	Stabilization - 1996 (Source – PA DCED)	
	Engineering	g, construction, funding strategy
\$945,000	Rehabilitation Engineering, construction, environmental	
	(Sources)	
	\$595,000	FHWA Enhancements
	\$340,000	FHWA National Covered Bridge
	\$ 10,000	Rockwood Area Historic Society
\$1.035 M	<u>TOTAL</u>	

Second National Covered Bridge Conference, Dayton Ohio 2013.

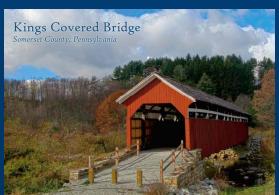
Technical Team

Prime Consultant – Simone Collins Inc. **Structural Engineer** – Samer Petro (formerly at Gannett Fleming Inc. now at Herbert Rowland Grubic) Historic Consultant – Dr. Emory Kemp **Stabilization Engineer** – DCF Engineering Inc. **Scientific Investigation** – Forest Products Laboratory **Stabilization Contractor** – Arnold M. Graton Assoc. **Rehabilitation Contractor** – Allegheny Restoration Surveyor – Paul C. Rizzo Inc. **Permit Assistance** - Somerset Conservation District.

Project Partners

Client – Southern Alleghenies Conservancy **Funding Partner** – Federal Highway Administration **Funding Partner** – Pennsylvania Commonwealth (PennDOT and DCNR) **Funding Partner** – Rockwood Area Historic Society **Funding Partner** – Somerset County **Project Administration** – PennDOT **Ultimate Owner** – Middlecreek Township **Steward / Donor** – King Family







William Collins, RLA Simone Collins Landscape Architecture 119 East Lafayette Street Norristown, PA 19401 610 239 7601 wcollins@simonecollins.com



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