

Concrete Pavement Thickness Design & Slab Geometry

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Agenda

- Review of Pavement Design History & Pavement Types
- Distresses Related to Pavement/Slab Geometry
- Compare AASHTO 93 vs Pavement ME Designs
- Incorporating Slab Geometry into Design Tools
- Using Slab Geometry to Control Cracking Mechanisms
 - Thickness
 - Joint Spacing
 - Widened Lanes
- Additional Design Considerations and Jointing

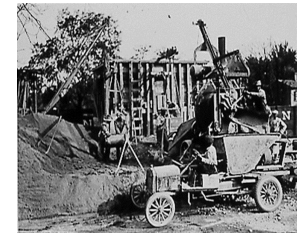


In The Beginning...



Early Concrete Pavement Details

- The first concrete pavements/slabs were:
 - $\approx 6"$ thick... no real structural design
 - 6' to 8' slabs
 - No crack control joints or dowels/steel



Design Challenge | Solution

- Vehicles Speeds Increased
- Loads Increased
- People Noticed Joint Roughness & Wanted to Maximize Production to Minimize Cost | Minimize Construction Joints



CONCRETE PAVING SOLUTIONS USING CONVENTIONAL CONCRETE

Jointed Plain Concrete Pavement (JPCP)

Jointed Reinforced Concrete Pavement (JRCP)

Continuously Reinforced Concrete Pavement (CRCP)

Shorter slabs w/ dowels & aggregate interlock to transfer loads

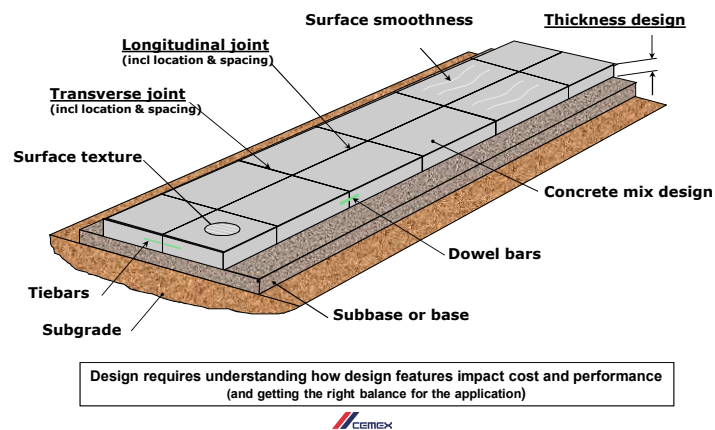
or

Longer (than JPCP) jointed w/ dowels to transfer loads

Continuously reinforced to control crack width

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JOINTED PLAIN CONCRETE PAVEMENTS (JPCP) – Key Design Items



Design also Requires an Understanding of How a Concrete Pavement Fails...

Structural Distress – the ability to carry traffic

Cracking (dominant)

Joint Faulting (dominant)

Functional Distress – the ability to serve the user comfortably

Rough ride (IRI) (mainly due to cracking and faulting)

Insufficient Texture/friction (address through maintenance)

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Rigid Pavement Design Tools/Methods



AASHTOWare Pavement ME
(previously known as DARWin-ME and MEPDG)

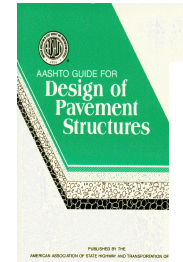
AASHTO, Guide for Design of Pavement Structures 1993



AASHTO 93
(software as ACPA WinPAS)



WinPAS 12



AASHTO, Guide for Design of Pavement Structures 1993



WinPAS 12

AASHTO 93 / WinPAS



Equivalent Single Axle Loads (ESALs)

- **ESAL = # of 18 kip (8,165 kg) equivalent single axles needed to cause same "response"**
 - Because pavement responses are different for concrete and asphalt, ESALs are different for the same exact traffic loading... **ESAL ≠ traffic**
 - ESALs depends on thickness, among other things
- Flexible ESALs generally about 1/3 less than rigid ESALs for highway-type traffic; NEVER COMPARE RIGID & FLEXIBLE ESALs

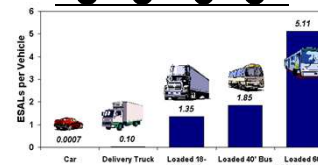
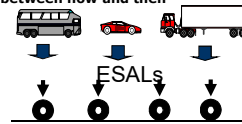


TRAFFIC IS THE MAIN SOURCE OF DAMAGE FOR PAVEMENTS

The Magnitude of Damage Depends on Vehicle Number, Type, and Load

Equivalent Single Axle Loads (ESALs)

- Assumes traffic is only 18,000 lbs single axles
- Conversion of trucks to ESALs is empirical
 - Based on field test conducted 50 years ago
 - Traffic conditions significantly changed between now and then



Load Spectrum

- Consider traffic composed of axles w/ different weights
- Required inputs:
 - Number of trucks
 - Axle load spectrum
 - Function of roadway type



Single Axles		Tandem Axles		Tridem Axles	
Axle load, kips	Axles / 1000 trucks	Axle load, kips	Axles / 1000 trucks	Axle load, kips	Axles / 1000 trucks
26	0.07	44	1.16		
24	1.6	40	7.76		
22	2.6	36	36.79		
20	6.63	32	54.76		
18	16.61	28	44.43		
16	23.88	24	30.74		
14	47.76	20	45		
12	116.76	16	59.25		
10	142.7	12	91.15		
8	233.6	8	47.01		



1986-93 JPCP AASHTO 93 Equation

$$\begin{aligned} \text{Log}(ESAL) = & Z_R * s_o + 7.35 * \text{Log}(D+1) - 0.06 + \frac{\text{Log} \left[\frac{\Delta PSI}{4.5-1.5} \right]}{1 + \frac{1.624 * 10^7}{(D+1)^{8.46}}} \\ & + (4.22 - 0.32 * p_i) * \text{Log} \left[\frac{S'_c * C_d * (D^{0.75} - 1.132)}{215.63 * J * \left[D^{0.75} - \frac{18.42}{(E_c/k)^{0.25}} \right]} \right] \end{aligned}$$

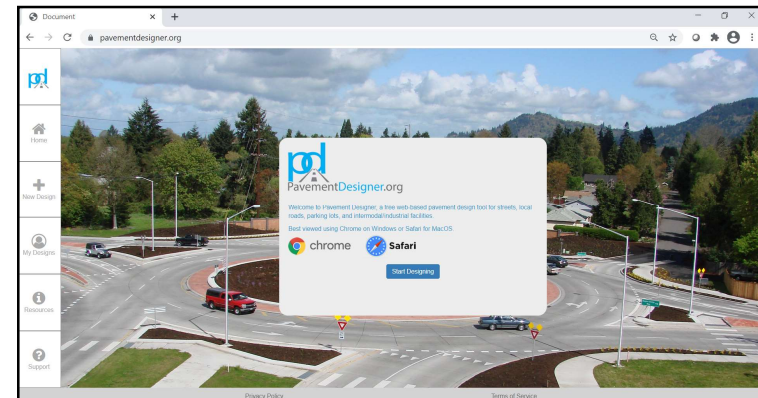
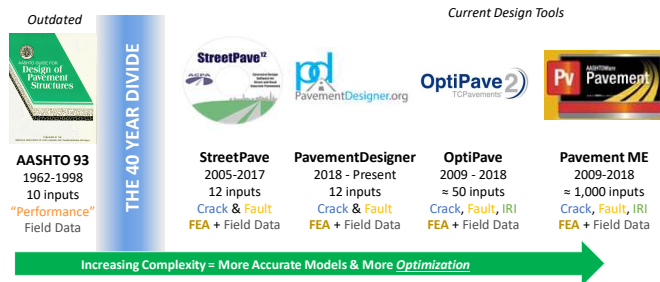
Standard Normal Deviate → Z_R
 Overall Standard Deviation → s_o
 Traffic → $\text{Log}(ESAL)$
 Change in Serviceability → ΔPSI
 Thickness → D
 Terminal Serviceability → p_i
 Modulus of Rupture → S'_c
 Drainage Coefficient → C_d
 Load Transfer → J
 Modulus of Elasticity → E_c
 Modulus of Subgrade Reaction → k

WHAT DO DESIGNERS FOCUS ON?

WinPAS Makes it Easy!

AASHTO 93 Slab Geometry involved => Thickness

Concrete Pavement Design Methodologies



Industry Developed Methods
PavementDesigner.org



PavementDesigner.org Background



A free tool designed to simplify concrete pavement design for:

- Parking lots
- Roadways (JPCP, RCC, CRCP, Overlays Unbonded & Bonded)
- Industrial / Intermodal yards (Forklifts & Specialty Equipment)

Uses More Accurate Traffic Inputs

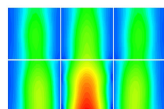
PD.org Slab Geometry => Thickness & Joint Spacing



MEPDG / DARWin-ME / AASHTOWare Pavement ME

Pavement ME Design

- Not “perfect” & not intended to be a “final” product
- Complex and relatively costly
- Primarily for high volume roadways



Mechanistic Calculation of Responses



Empirical Tie to Ground



Pavement Performance Prediction



Sounds Easy Enough, Right?

$$Fault_t = \sum_{i=1}^n \Delta Fault_i$$

$$\Delta Fault_i = C_{34} * (FAULTMAX_{i-1} - Fault_{i-1})^2 * DE_i$$

$$FAULTMAX_i = FAULTMAX_0 + C_7 * \sum_{j=1}^m DE_j * \log(1 + C_5 * 5.0^{EROD})^{C_6}$$

$$FAULTMAX_0 = C_{12} * \delta_{curving} * \left[\log(1 + C_5 * 5.0^{EROD}) * \log\left(\frac{P_{200} * WetDays}{p_i}\right) \right]^{C_4}$$

$$\sigma_0 = \frac{E_{PCC} \Delta \epsilon_{gr}}{2(1 - \mu_{PCC})}$$

$$IRI = IRI_1 + C1 * CRK + C2 * SPALL + C3 * TFAULT + C4 * SF$$

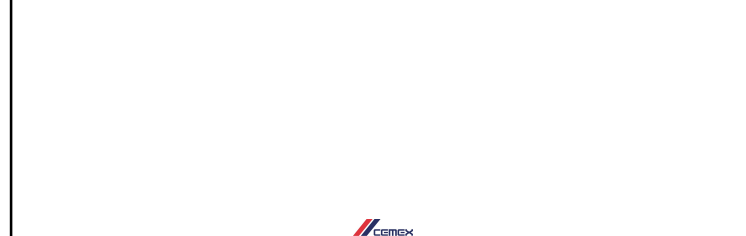
$$SCF = -1400 + 350 * AIR\% * (0.5 + PREFORM) + 3.4 f_c * 0.4 - 0.2 (FTCYC * AGE) + 43 h_{PCC} - 536 WC_Ratio$$

$$CW = \max \left(L * \left(\epsilon_{shr} + \alpha_{PCC} \Delta T_c - \frac{C_2 f_g}{E_{PCC}} \right) * 1000 * CC, 0.001 \right)$$



MEPDG / DARWin-ME / AASHTOWare Pavement ME

Concrete Pavement Design Options

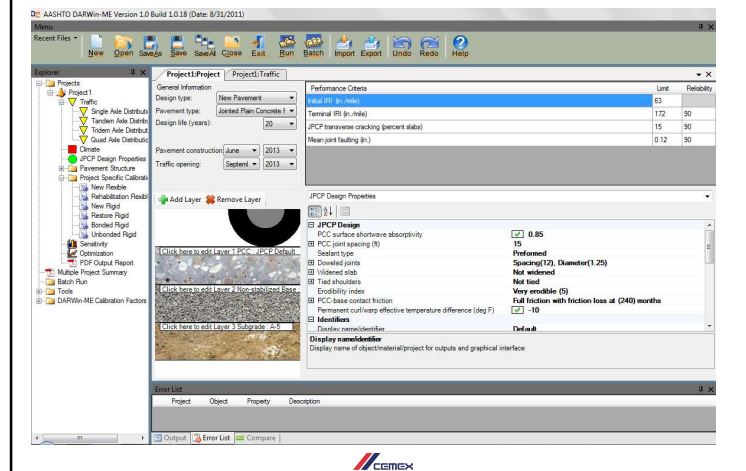


Pavement ME's Concrete Pavement Designs

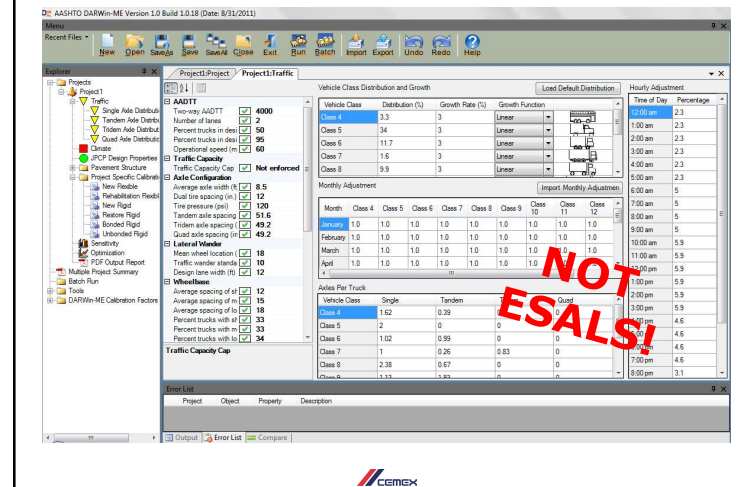
- New Pavement
 - Jointed Plain Concrete Pavement (JPCP)
 - Continuously Reinforced Concrete Pavement (CRCP)
- Overlay
 - Bonded PCC over JPCP or CRCP
 - Unbonded JPCP or CRCP over JPCP or CRCP
 - JPCP over AC
 - CRCP over AC
 - SJPCP over AC
- Rehabilitation



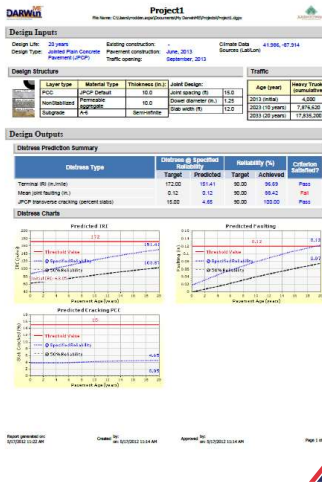
Pavement ME Inputs...



EXACT Traffic Inputs...



Pavement ME Outputs...



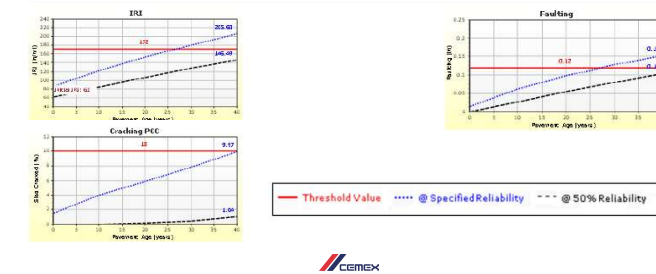
Pavement ME Performance Outputs

Design Outputs

Distress Prediction Summary

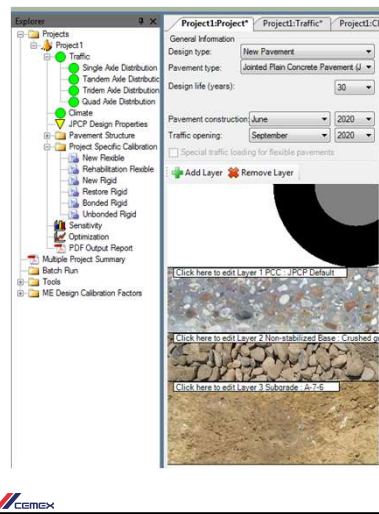
Distress Type	Distress @ Specified Reliability		Reliability (%)		Criterion Satisfied?
	Target	Predicted	Target	Achieved	
Terminal IRI (in/mile)	172.00	205.60	90.00	70.99	Fail
Mean joint faulting (in)	0.12	0.15	90.00	66.92	Fail
JPCP transverse cracking (percent slabs)	10.00	9.97	98.00	98.03	Pass

Distress Charts

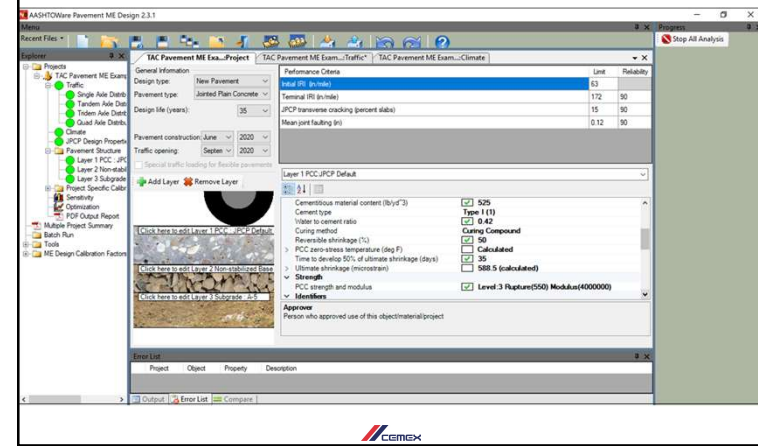


Jointed Plain Concrete Pavement (JPCP)

- JPCP Design Process
 - General Info and Performance Criteria
 - Traffic Details
 - Climate
 - Characterizing Pavement Structure
 - JPCP Design Properties



JPCP – Characterizing Pavement Structure



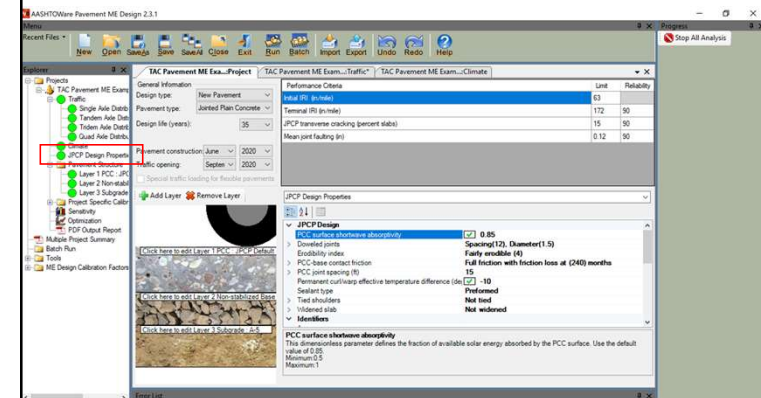
JPCP – Pavement Structure – PCC Materials

PCC		
Poisson's ratio	<input checked="" type="checkbox"/>	0.2
Thickness (in)	<input checked="" type="checkbox"/>	11
Unit weight (pcf)	<input checked="" type="checkbox"/>	150
Thermal		
PCC coefficient of thermal expansion (in/in/deg F x 10 ⁻⁵)	<input checked="" type="checkbox"/>	5.5
PCC heat capacity (BTU/lb-deg F)	<input checked="" type="checkbox"/>	0.28
PCC thermal conductivity (BTU/hr-ft-deg F)	<input checked="" type="checkbox"/>	1.25
Mix		
Aggregate type		Limestone (1)
Cementitious material content (lb/yd ³)	<input checked="" type="checkbox"/>	525
Cement type		Type I (1)
Water to cement ratio	<input checked="" type="checkbox"/>	0.42
Curing method		Curing Compound
Reversible shrinkage (%)	<input checked="" type="checkbox"/>	50
PCC zero-stress temperature (deg F)		Calculated
Time to develop 50% of ultimate shrinkage (days)	<input checked="" type="checkbox"/>	35
Ultimate shrinkage (microstrain)		588.5 (calculated)
Strength		
PCC strength and modulus	<input checked="" type="checkbox"/>	Level 3 Rupture(550) Modulus(4000000)

Let's Break it Down



JPCP – Design Properties



JPCP – Design Properties

JPCP Design		
PCC surface shortwave absorptivity	<input checked="" type="checkbox"/>	0.85
Doweled joints		Spacing(12), Diameter(1.5)
Erodibility index		Fairly erodible (4)
PCC-base contact friction		Full friction with friction loss at (240) months
PCC joint spacing (ft)		15
Permanent curl/warp effective temperature difference (deg F)	<input checked="" type="checkbox"/>	-10
Sealant type		Preformed
Tied shoulders		Not tied
Widened slab		Not widened

Let's Break it Down



JPCP – Design Properties

JPCP Design		
PCC surface shortwave absorptivity	<input checked="" type="checkbox"/>	0.85
Doweled joints		Spacing(12), Diameter(1.5)

- SSA
- Doweled Joints
- Diameter
- Spacing
- 0.85 (Default and semi-constant)
- Typically used if thickness > 8 in
- Often depends on thickness
 - 1 inch for 8 inches or less thickness
 - 1.25 inches for 8 – 10 inches thickness
 - 1.5 inches for >10 inches
- 12 inches is most common



JPCP – Design Properties

Erodibility index	Fairly erodible (4)
> PCC-base contact friction	Full friction with friction loss at (240) months
> PCC joint spacing (ft)	15

- Erodibility
- Base Friction
- **Joint Spacing**
- Depends on soil conditions
- Good defaults
- Typical range = 12 – 20 ft



JPCP – Design Properties

Permanent curl/warp effective temperature difference (deg)	✓ -10
Sealant type	Preformed
> Tied shoulders	Not tied
> Widened slab	Not widened

- Curl/Warp Temp.
- Sealant Type
- Tied Shoulder
- **Widened Slab**
- -10°F (Good default)
- Preformed or Other (none, liquid, silicone)
- Project dependent
- Project dependent

**Pavement ME Slab Geometry Inputs include
Thickness, Joint Spacing, Lane Width**



Summary of Unique JPCP Critical Inputs

- Performance Criteria
 - IRI, Cracking, Faulting
- **Thickness**
- **Joint Spacing**
- **Lane Width**
- **Shoulder Type**
- **Dowel Design**
- PCC Strength
- PCC Modulus
- Coef. of Thermal Exp.
- Curing Method
- Base Erodibility
- Mix Design (Cement type, w/cm, etc.)

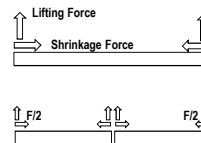
BOLD => Inputs Related to Slab Geometry



SHORT JOINT SPACING IMPROVES JPCP PERFORMANCE

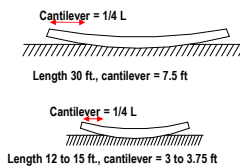
Reduces Shrinkage Force

- Curling & warping is due to the differential drying and thermal shrinkage at the slab surface
- Shorter slabs have less length, which means reduced curling



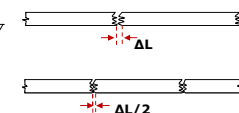
Reduces Environmental Stress

- ~1/4 of slab length is cantilever
- Reducing unsupported length reduces the bending stress
- Reducing length reduces uplift and improves smoothness

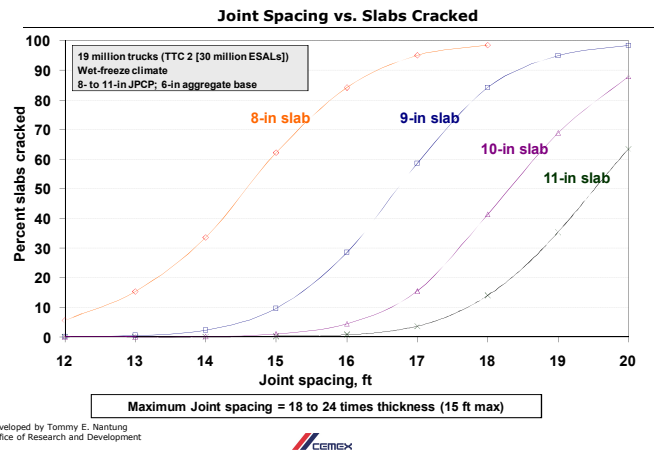


Improves Load Transfer

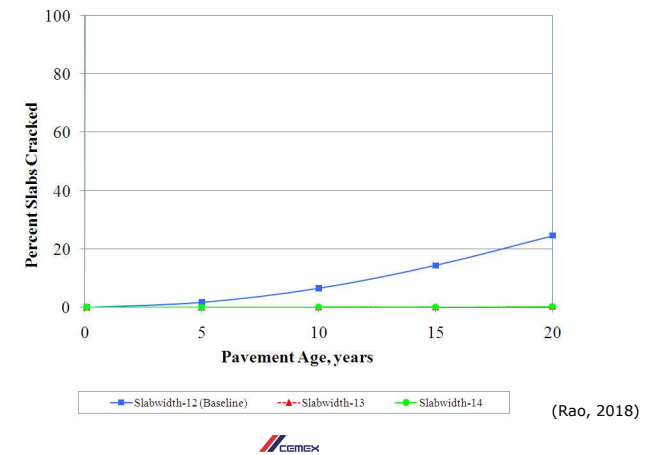
- Shorter slabs have smaller joint/crack opening
- Agg. Interlock stronger for tighter cracks
- High load transfer results in less stress in concrete



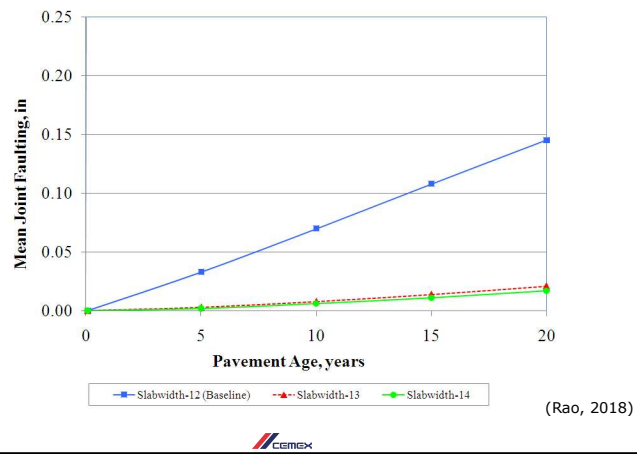
SHORT JOINT SPACING REDUCES SLAB CRACKING



Engineering Solutions – Widened Slab Example



Engineering Solutions – Widened Slab Example



Engineering Solutions – Widened Slab Example

- Widening the slab reduces longitudinal edge midpanel stresses but this could increase stresses in other locations not considered in Pavement ME
- With 14 ft wide slab there is a much higher risk of longitudinal cracking due to increased stresses at interior transverse joint edge locations



(Rao, 2018)

CERMEX

Highway Design Problem

Highway Design Problem

DESIGN SUMMARY REPORT FOR
JOINTED PLAN CONCRETE PAVEMENT (JPCP)

DATE CREATED: Thu Oct 24 2019 10:10:11 GMT-0500 (Central Daylight Time)

Project Description
Project Name: ARDOT - I-30 and Ramps Concrete Pavement Design Analysis - Optimal
Designers Name: undefined
Project Description: undefined

Design Summary
Recommended Design Thickness: 8.50 ft. (8.50 ft.)
Calculated Minimum Thickness: 8.43 ft. (8.43 ft.)
Maximum Joint Spacing: 15 ft. (15 ft.)

Pavement Structure
SUBGRADE
User Defined Compacted k Value of Subgrade: 180 pci
Layer Type: 180 pci
Layer Thickness: 180 pci
SUBGRADE

CONCRETE
Design Day Strength: 4500 psi
Modulus of Elasticity: 3000000 psi
Edge Support: No
Number of Reinforcing Bars: 0
SUBGRADE
Calculated MGS Value: 4.302 psi

Project Level
TRAFFIC
Design Year: 20 years
Major Annual Traffic: 7,000
Traffic Growth Rate: 1.5% per year
Directional Distribution: 50%
Design Lane Distribution: 50%

GLOBAL
Reliability: 95%
% States Created at End of Design Life: 5%
Avg Traffic Day in Design Lane Over the Design Life: 2,000
Total Traffic in Design Lane Over the Design Life: 10,000,000

Report generated on: 10/24/2019 10:10 AM
Version: 2.2.0.0
Created by: 10/24/2019 10:10 AM
Approved by: 10/24/2019 10:10 AM
Page 1 of 10

Additional Design Considerations Related to Slab Geometry

- Dowels
 - Dowel Spacing
 - Dowel Bar Diameter
- Edge Support
- Tie Shoulders
- Jointing Layouts
- ...CRCP Design Properties



Top 10 ME Design Most Sensitive

1. Concrete Flexural Strength at 28-Days
2. Concrete Thickness
3. Surface Shortwave Absorptivity (SSA)
4. Joint Spacing
5. Concrete Modulus of Elasticity at 28-Days
6. Design Lane Width with a 14 ft (4.3 m) Widened Slab
7. Edge Support via Widened Slab
8. Concrete Thermal Conductivity
9. Concrete Coefficient of Thermal Expansion (CTE)
10. Concrete Unit Weight

Project 1-47

Sensitivity Evaluation of MEPDG Performance Prediction

Final Report

Prepared for the
NATIONAL COOPERATIVE HIGHWAY RESEARCH PROGRAM
TRANSPORTATION RESEARCH BOARD
OF
THE NATIONAL ACADEMIES

Charles W. Schwartz
Rui Li
University of Maryland
College Park, MD
Song Hwan Kim
Haili Ceylan
Karthiraman Gopalakrishnan
Jowa State University
Ames, IA

December 2011

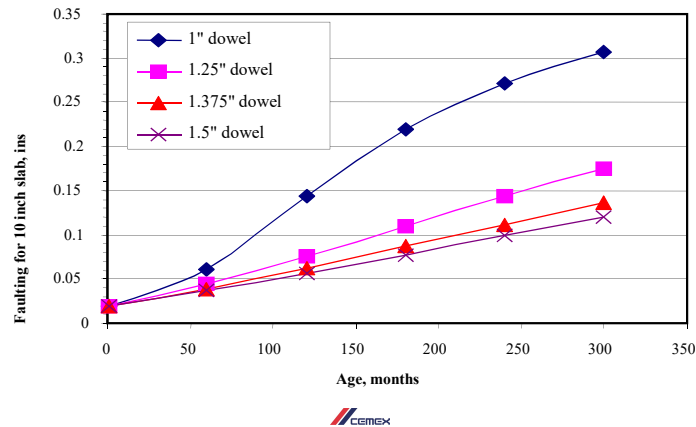
http://onlinepubs.trb.org/onlinepubs/nchrp/docs/NCHRP01-47_FR.pdf

Engineering Solutions - Faulting

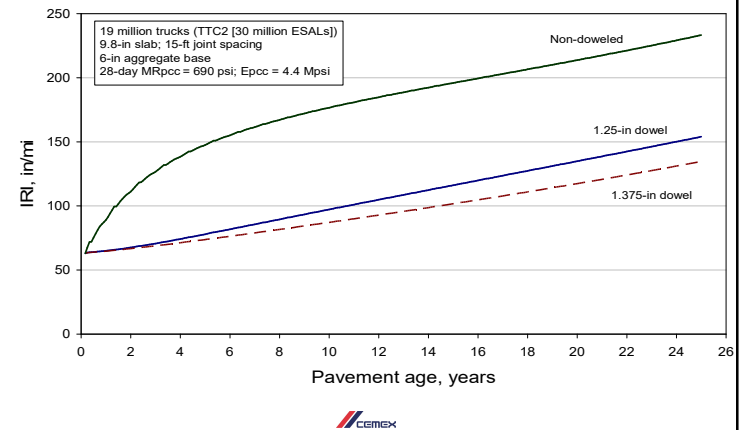
- Improve Mechanical LT
 - Increase Dowel Size
 - Decrease Dowel Spacing
- Decrease Joint Spacing
- Increase Width of Lanes
- Reduce Underlying Layer Erosion
 - Increase Erodibility Index
 - Decrease Joint Spacing
- Reduce Thickness
 - Only if Cracking is Passing



Sensitivity of JPCP Faulting to Dowel Diameter



Sensitivity of JPCP IRI Sensitivity to Dowels



Sounds Easy Enough, Right?

$$Fault_m = \sum_{i=1}^m \Delta Fault_i$$

$$\Delta Fault_i = C_{34} * (FAULTMAX_{i-1} - Fault_{i-1})^2 * DE_i$$

$$FAULTMAX_i = FAULTMAX_0 + C_7 * \sum_{j=1}^i DE_j * \log(1 + C_5 * 5.0^{EROD}) * C_4$$

$$FAULTMAX_0 = C_{12} * \delta_{swelling} * \left[\log(1 + C_5 * 5.0^{EROD}) * \log\left(\frac{P_{200} * WetDays}{P_i}\right) \right]^{C_4}$$

$$\sigma_0 = \frac{E_{PCC} \Delta \epsilon_{tot}}{2(1 - \mu_{PCC})}$$

$$IRI = IRI_1 + C1 * CRK + C2 * SPALL + C3 * TFAULT + C4 * SF$$

$$SCF = -1400 + 350 * AIR\% * (0.5 + PREFORM) + 3.4 f'c * 0.4 - 0.2 (FTCYC * AGE) + 43 h_{PCC} - 536 WC_Ratio$$

$$cw = \max \left(L * \left(\epsilon_{shr} + \alpha_{PCC} \Delta T_s - \frac{c_2 f'c}{E_{PCC}} \right), 1000 * CC, 0.001 \right)$$

Continuously Reinforced Concrete Pavement (CRCP)

Resources: Check out crcpavement.org for more!

April 2011

CONTINUOUSLY REINFORCED CONCRETE PAVEMENT

Design & Construction Guidelines

Rasmussen et al. (2011)

August 2016

CONTINUOUSLY REINFORCED CONCRETE PAVEMENT MANUAL

Guidelines for Design, Construction, Maintenance, and Rehabilitation

Roesler et al. (2016)

April 2013

Continuously Reinforced Concrete Pavement: Design Using the AASHTOWare Pavement ME Design Procedure

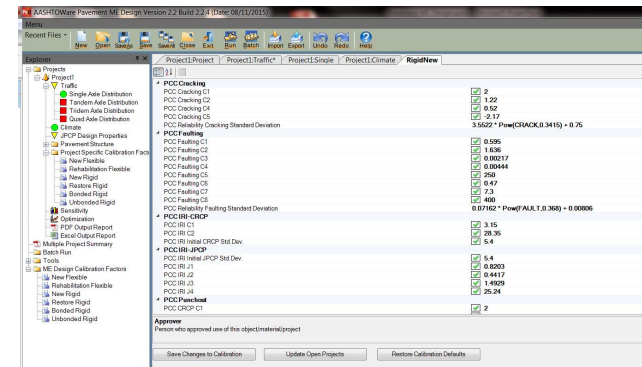
Roesler & Hillier (2013)

CRCP Design Properties Related to Slab Geometry

- Lane Width
- Crack Spacing (Dependent on Steel Design & Base Friction)
- Steel Design
 - % Steel
 - Bar Diameter
 - Bar Depth
- Base Friction Coefficient
- Shoulder Type



Pavement ME Allows Agencies To Develop And Use Local Calibration Coefficients



You can save your local calibration coefficients as default or restore the national as default at one click



Local Calibration Examples

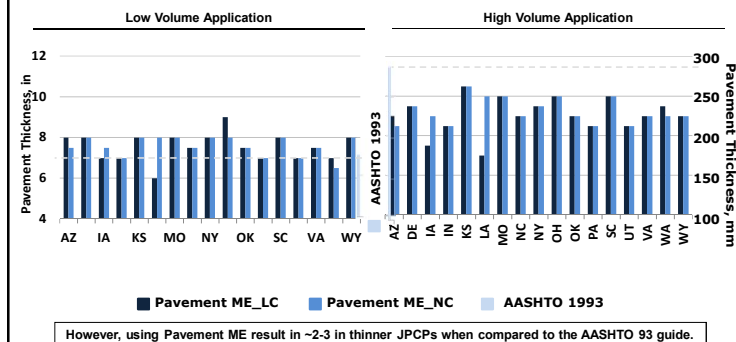
- Indiana DOT:
 - Changed JPCP IRI J3 from 1.4929 to 1.05 because it was too sensitive to it
- Ohio DOT:
 - Changed JPCP IRI calibrations

Calibration Coefficient	Default (national)	Ohio
PCC IRI J1	0.8203	0.82
PCC IRI J2	0.4417	3.7
PCC IRI J3	1.4929	1.711
PCC IRI J4	25.24	5.703
PCC IRI JPCP Standard Deviation	5.4	5.4

- Many states at this point are working on or have completed local calibrations.



Local Calibration Result In 1/2-In Or Less Difference In Required Thickness Vs. National Calibration



(Mu, 2017)



Simpler ME Option: Design Tables

		Average Annual Daily Truck Traffic (AADTT) - 25 Year Pavement Design					
		Collector		Minor Arterial		Major Arterial	
		250	500	1,000	1,500	2,500	3,000
Subgrade Strength (CBR-%)	PCC	180 mm PCC 200 mm Gravel A	180 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm
	ICMA	40 mm SP 12.5 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 80 mm SP 19 150 mm Gravel A 400 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 450 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 450 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 450 mm Gravel B	200 mm
	PCC	180 mm PCC 200 mm Gravel A	180 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm
	ICMA	40 mm SP 12.5 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 400 mm Gravel B	200 mm
	PCC	180 mm PCC 200 mm Gravel A	180 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm PCC 200 mm Gravel A	200 mm
	ICMA	40 mm SP 12.5 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 350 mm Gravel B	40 mm SP 12.5 FC1 80 mm SP 19 150 mm Gravel A 400 mm Gravel B	200 mm
Concrete Slab and Joint Properties		No dowels Slab length = 4 m Tied shoulder/curb *		32M Dowel bars, 300 mm spacing Slab length = 4.5 m Tied shoulder/curb *		32M Dowel bars, 300 mm spacing Slab length = 4.5 m Tied shoulder/curb *	



Conclusions:



- Slab Geometry is KEY to Optimizing Pavement Designs
- Thickness is not the ONLY Slab Geometry that Improves Performance
- Shorter Joint Spacings & Widened Lanes Improve Pavement Performance
- Improvements in Design Tools, such as Pavement ME, have allowed Designers to Utilize all aspects of Slab Geometry to Yield more Economical and Better Performing Concrete Pavements



125 Yrs of Success and Performance



Acknowledgements & References

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- Rao, Shreenath 2018. Pavement ME Design – 3M Edition – Myths, Misuses, & Misconceptions, Presentation to 2018 CO/WY ACPA Meeting, 3-22-2018.
- Donahue, John & Jason Bloomberg 2018. *AASHTO Pavement ME Design Web Application JPCP Module*, Presentation to MO/KS ACPA's 38th Annual PCCP Conference, 2-20-2018.



Resources

- NCHRP 1-37 MEPDG Home: <http://onlinepubs.trb.org/onlinepubs/archive/mepdg/guide.htm>
- Recorded Webinars: <https://www.fhwa.dot.gov/pavement/dgit/aashtoware.pdf>
- North American Usergroup Summary Page: <http://www.pooledfund.org/Details/Study/549>
- ME Design Help: http://www.me-design.com/MEDesign/data/HTML%20Help/US/index.html?design_inputs_1.htm
- Application Library: <http://apps.acpa.org/>



Resources

- Some States with Pavement ME User Guides
 - Michigan: https://www.michigan.gov/documents/mdot/MDOT_Mechanistic_Empirical_Pavement_Design_User_Guide_483676_7.pdf
 - Colorado: <https://www.codot.gov/business/designsupport/matgeo/manuals/pdm/2017-m-e-pavement-design-manual/chapter-1.pdf>
 - Indiana: http://www.in.gov/indot/design_manual/files/Ch304_2013.pdf
 - Arizona: https://apps.azdot.gov/ADOTLibrary/publications/project_reports/PDF/AZ606.pdf
 - Virginia: http://www.virginiadot.org/VDOT/Business/asset_upload_file108_3638.pdf
 - Utah: <https://www.udot.utah.gov/main/uconowner.gf?n=20339215312776663>

